

Lynx Starter Dam 2019 Site Investigation Report

Myra Falls Mine Wood Project # NX14001C1

Prepared for:



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Prepared for:

Nyrstar Myra Falls Ltd. Campbell River, BC

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30 April 2020

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1.0 Introduction

This factual data report summarizes the field program carried out for the 2019 Lynx starter dam site investigation at Nyrstar's Myra Falls Mine, located in Strathcona-Westmin Park approximately 55 km south of Campbell River on Vancouver Island, BC. The purpose of the site investigation was to collect geotechnical information for the Lynx starter dam after Wood (formerly Amec Foster Wheeler Environment & Infrastructure) identified a zone of nonconformance fill material in the southwest corner of the existing dam toe during the construction quality control activities for the 362.5 m elevation raise of Lynx TDF Dam in July 2017. The subject zone, designated as Panel 5, did not conform to the construction specifications. Wood reviewed existing information from previous site investigations completed at the Lynx TDF Dam site and record reporting from Wood and Klohn Crippen Berger (Klohn). A geotechnical site investigation was recommended to characterize the extent of the nonconformance fill materials observed during construction.

The proposed site investigation program was detailed in a letter to Nyrstar Myra Falls Mine dated August 27, 2019. The site investigation was completed in late 2019. This report contains site investigation factual information and presents a summary of the findings of the site investigation.

The primary focus of the site investigation was to identify nonconforming fill materials within the starter dam. An additional objective was to gain a better understanding of the foundation materials and the materials used during the early raises of the Lynx TDF Dam downstream shell construction prior to 2013. The data gained from the site investigation was intended to update the Lynx TDF stability model if warranted.

2.0 Scope of Work

Field activities were conducted from October 30 to November 5, and November 25 to December 3. The scope of the geotechnical site investigation included:

- Drilling paired sets of boreholes within 2 m of each other using instrumented Becker Penetration Testing (iBPT) for in situ strength testing followed by sonic drilling for sample recovery, at four locations along the dam crest at elevation 362.5 m (BH19-02/iBPT19-02, BH19-03/iBPT19-03, BH19-05/iBPT19-05&05B, and BH19-06/iBPT19-06);
- Drilling of two sonic boreholes (BH19-01 and BH19-04) along the dam centerline at elevation 365.1 m on the western side of the dam, to install slope indicators extending well below (approximately 10 m) the starter dam fill;
- Installing three vibrating wire piezometers (VWPs) in borehole BH19-02 (30.5 m depth), BH19-03 (31 m depth) and BH19-06 (35.5 m depth) at depths corresponding with the interpreted base of starter dam fill;
- Conducting two arrays of multi-channel analysis of surface waves (MASW); and
- Conducting two downhole seismic tests (DST) at two selected geotechnical boreholes.

Shallow refusal was encountered during the drilling of the iBPT holes at elevations corresponding approximately with the top of the starter dam (between about 23 and 24 m depth), so Standard Penetration Tests (SPTs) were completed below these depths in the sonic holes to provide some measure of in situ density of the starter dam fills. An additional open Becker hole was also attempted (iBPT19-05B) in an unsuccessful effort to penetrate the refusal layer.

The locations of the boreholes and geophysics lines are shown on a plan view in Figure 1, and on section views in Figure 2. The section views in Figure 2 correspond with the section lines drawn on Figure 1.



The iBPT results are presented in Appendix A and the Geophysical testing report is included in Appendix B. Amalgamated borehole logs are presented in Appendix C, and are identified as simply BH19-01 to BH19-06, except for the iBPT data from iBPT19-05B which is presented in a separate log. The RST instrument calibration documents and site investigation photographs are presented in Appendix D and Appendix E, respectively.

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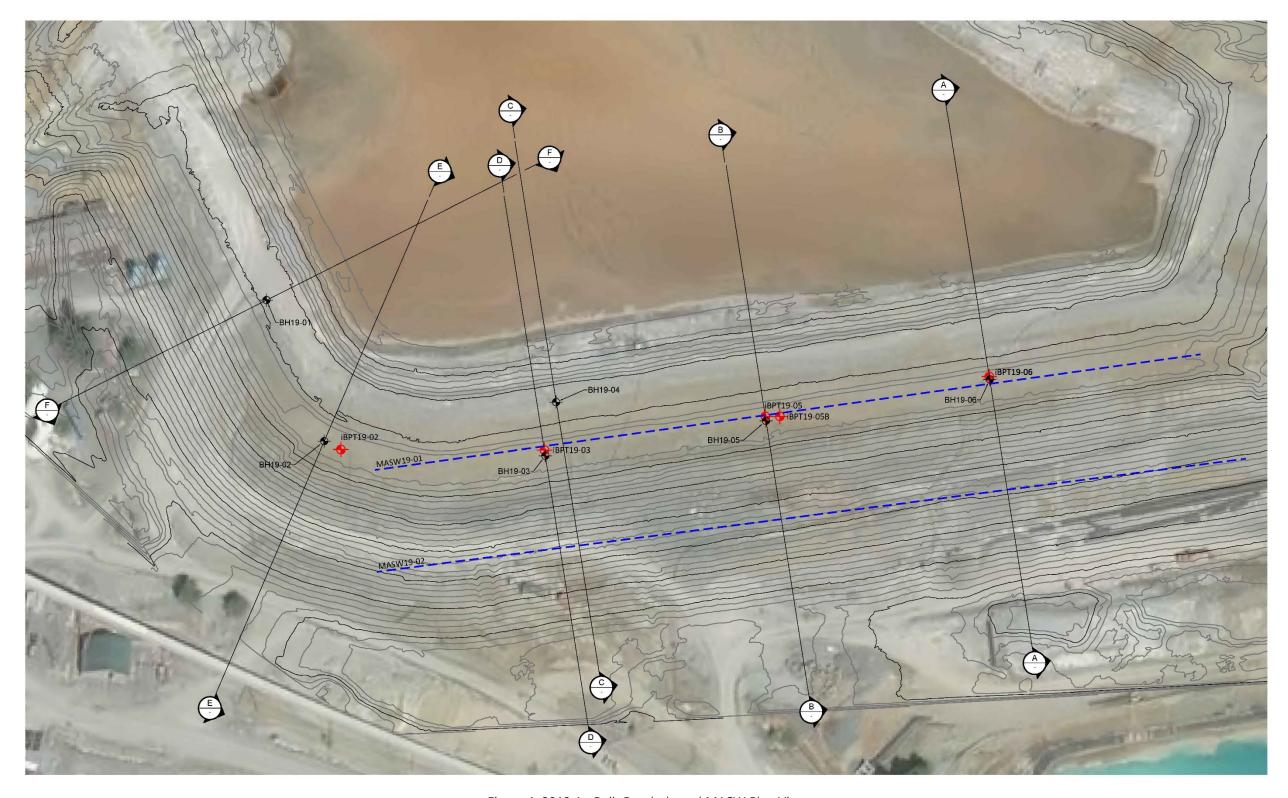


Figure 1: 2019 As-Built Borehole and MASW Plan View

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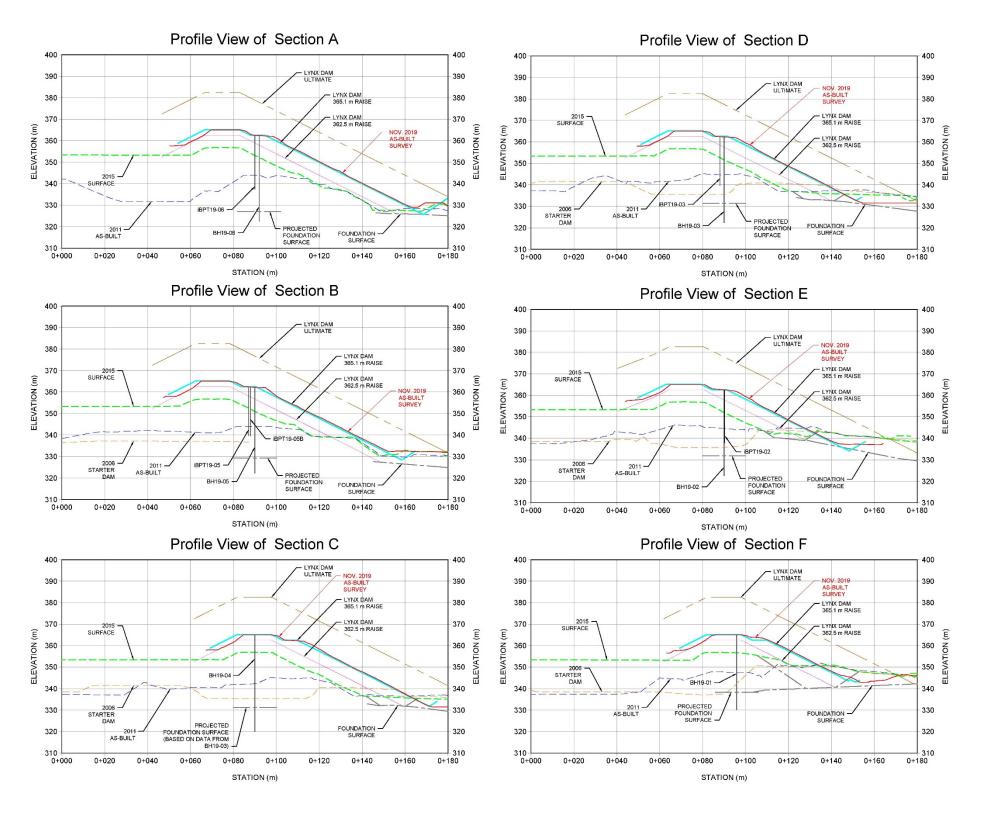


Figure 2: 2019 As-Built Borehole Section View

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3.0 Investigation Methodologies

The site investigation program was completed in two stages. The first stage consisted of completion of the iBPT boreholes conducted from October 30 to November 5, 2019. The second stage consisted of sonic drilling and instrumentation installation which was completed November 25 to December 3, 2019. Site safety orientations provided by Nyrstar, were conducted for each stage of site investigation for ConeTec, Mudbay Drilling and Wood personnel on October 30 and November 25, 2019.

3.1 iBPT Drilling

Five iBPT boreholes were completed along the 362.4 m downstream bench of the Lynx TDF from October 30 to November 5, 2019. ConeTec Investigation Ltd. was subcontracted by Nyrstar and coordinated by Wood, to conduct the iBPT profiles. The iBPT borehole drilling was completed under ConeTec's direction and supervision.

Two different drilling methods were used – closed-end and open-end. The closed-end method involves the use of the Becker rig to drive (with a hammer) closed-end casing which displaces the soil ahead of the bit. The open-end casing method is similar, except that compressed air is delivered through the annulus of the double wall casing, which forces drill cuttings to rise from the bit face through the center of the casing into a cyclone at the ground surface. With both methods the rig is instrumented to measure the resultant number of blows per foot, the number of blows per minute, and the pressure within the bounce chamber at the top of the hammer.

All holes were drilled to refusal which occurred at depths between 22.8 m (74.9 ft) and 25.2 m (82.7 ft) below the ground surface (362.4 m downstream bench). Based on estimates from record drawings, the refusal was met just below the crest of the starter dam. All holes refused at similar depths, indicating a very dense or possibly cemented layer. Refusal was met at depths much less than the target depth of 40 m. After refusal at 22.9 m, iBPT19-05 was advanced open-ended to refusal at 23.4 m depth. The hole was then re-drilled (iBPT19-05B) at an offset of 5 m, and encountered refusal at a similar depth. A copy of the iBPT report by ConeTec is presented in Appendix A. All iBPT holes were backfilled with cement-bentonite grout upon completion. A summary of the iBPT boreholes is presented below in Table 1 and the results are presented graphically in the borehole logs in Appendix C.

Table 1: iBPT Borehole Summary Table

ID	Northing	Easting	Start Date	Final Date	Ground Elevation (m)	Refusal Depth (m)
iBPT19-02	5494652.8	311735.4	04-Nov-2019	05-Nov-2019	362.4	24.7
iBPT19-03	5494652.6	311805.2	03-Nov-2019	04-Nov-2019	362.4	22.8
iBPT19-05	5494664.1	311880.9	01-Nov-2019	02-Nov-2019	362.4	22.9
iBPT19-05B	5494664.1	311875.9	02-Nov-2019	03-Nov-2019	362.4	23.0
iBPT19-06	5494676.8	311957.9	30-Oct-2019	31-Oct-2019	362.4	25.2



3.2 Sonic Drilling

The sonic drilling program started on November 25, 2019 and ended December 3, 2019. A summary of the sonic boreholes is shown in Table 2. Large Penetration Testing (LPT) was completed at 1.5 m intervals between the iBPT refusal depth and the bottom of the starter dam foundation. The LPT tests were completed to provide an indication of relative density of the materials within the starter dam below the iBPT refusal depth. A profile view of each completed borehole is presented on Figure 2, with the original ground surface projected beneath the starter dam based on interpretation of natural ground elevations from the sonic core logging.

Mudbay Drilling brought a rotary coring attachment to site to drill through the impenetrable layer in the case the sonic bit also met refusal, but the sonic bit was successful in penetrating the layer on which refusal was encountered.

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ID	Northing	Easting	Start Date	Completion Date	Collar Elevation (m)	Bottom Elevation (m)	Total Depth (m)	Comments
BH19-01	5494704	311709.9	25-Nov- 2019	26-Nov- 2019	365.1	328.5	36.6	SI casing installed
BH19-02	5494655.6	311729.7	27-Nov- 2019	28-Nov- 2019	362.4	322.8	39.6	VWP installed
BH19-03	5494650.7	311805.5	29-Vov- 2019	30-Nov- 2019	362.4	322.8	39.6	VWP installed
BH19-04	5494669	311809.2	26-Nov- 2019	27-Nov- 2019	365.1	319.4	45.7	SI casing installed
BH19-05	5494664.1	311880.9	1-Dec- 2019	2-Dec- 2019	362.5	322.9	39.6	No Instrument
BH19-06	5494677.8	311957.7	2-Dec- 2019	3-Dec- 2019	362.4	322.8	39.6	VWP installed

Table 2: As-Built Sonic Borehole Summary Table

The work was executed as follows:

- A tailgate safety meeting was held daily to discuss the anticipated hazards, personal protective equipment (PPE) required and hazard mitigation procedures prior to drilling;
- All boreholes, except BH19-01, were staked in the field by Mifflin Surveys. This work was done one day after the start of the investigation program. BH19-01 was located using the plan drawings and existing features in the field. The as-built location of this borehole was about 5 m away from the proposed location. Final borehole locations were determined based on field conditions and safety requirements. In general, the boreholes were laid out within a few meters from the proposed locations. The as-built coordinates were computed upon completion and presented in Table 2;
- A Wood representative logged the soil samples from the sonic cores based on the Modified Unified Soil Classification System (MUSCS). Details of the MUSCS and the borehole logs are presented in Appendix C;
- A photo log of representative core runs and LPT samples is included in Appendix E;
- Representative soil samples were collected from the sonic cores and placed in plastic bags for further laboratory testing. The name of the borehole and depth of the sample were labelled on the bag.
 Samples were packaged and transferred to the Wood site office at Myra Falls Mine;

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- Drilling continued until the termination depth was reached; and
- The drill holes were backfilled to the ground surface using cement bentonite grout mix.

Aerial imagery of borehole locations is provided on Figure 1; borehole cross sections are shown on Figure 2 and borehole coordinates are included in Table 1 and Table 2.

3.3 Geophysics Program

ConeTec personnel were on site to conduct the geophysical portion of the site investigation program. This geophysical program consisted of 2 two-dimensional MASW profiles and 2 DST profiles. The first MASW line, MASW19-01, was started on November 25 and completed on November 26. The second MASW line, MASW19-02, was started on November 27 and completed on November 28.

The DST19-01 and DST19-02 profiles were conducted in boreholes BH19-02 and BH19-03, respectively. The DST testing was conducted at 0.5 m intervals in the foundation soils and 1.0 m intervals in the dam fill materials. The DST profiles were started on November 29 and completed December 1. A summary table of the geophysical tests is presented in Table 3.

ID Start Start End End Completion **Comments Easting Northing Easting Northing Date** MASW19-01 5494646 311747.1 5494685.4 312030.1 26-Nov-2019 An excavator was used to remove MASW19-02 5494610.9 312045.7 311747.8 5494649.7 28-11-2019 large rocks & boulders along the line. BH19-02 DST completed 29 November 2019 BH19-03 DST completed 1 December 2019

Table 3: Summary Table of MASW Profile Lines and DST Profiles

3.4 Laboratory Program

The planned laboratory testing program was not completed since deleterious materials were not encountered in the boreholes.

4.0 Summary of Findings and Test Results

The following is a summary of the material properties encountered by the iBPT and Sonic drill holes, and the interpreted results of the geophysical survey. The encountered materials are generally grouped in three zones defined based on physical properties and visual classification of the drilled core samples. Each zone is described in the following subsections.

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4.1 Lynx TDF Dam - Downstream Shell

In general, Lynx tailings dam materials observed in the sonic cores consisted of fill materials composed of non ore-bearing rock and overburden soil produced during historical open pit and underground mining operations at the site, and removed from existing dumps on the slopes above the Lynx TDF. This material is generally coarse-grained angular gravel with some angular sand and trace of non plastic silts. The overburden material generally consists of excavated till-like materials and random mixture of gravel, sand, and fines in varying proportions. The silts within the overburden material are generally non plastic, whereas the silts/clays from the till-like materials have low plasticity. In some areas, a mixture of the mine waste and overburden fill materials may have been produced due to multiple handling during excavation and placement.

Waste rock fill materials within the downstream shell appear to have been placed with lift thicknesses ranging from 0.4 m to 0.6 m. Various lifts were easily identifiable based on moisture content, silt content, and varied coloration.

The relative density of the waste rock fill at individual boreholes was interpreted based on correlation with equivalent N_{60} values derived from the iBPT data. The relative density of the waste rock fill materials, based on correlation with the equivalent N_{60} values, ranges from 40% to 100% with average results ranging falling between 70% and 90%. Based on this data, the average density of the Lynx TDF dam waste rock fill can be generally classified as dense.

The geophysical site investigation program consisted of two MASW and two DST profiles. The program indicated that the waste rock fill materials are very dense. The waster rock fill average shear wave velocity (V_s) along the MASW19-01 ranges from 415 m/s to 657 m/s with an average of 497 m/s. The average velocity (V_s) at both DST locations ranges from 461 m/s to 751 m/s with an average of 596 m/s, which also indicates very dense soil conditions, based on Table 4.1.8.4.A in 2015 edition of the National Building Code of Canada (NBCC 2015).

4.2 Lynx TDF Dam - Starter Dam

The Lynx TDF starter dam materials consisted of waste rock fill materials, similar to those described in section 6.1. Waste rock fill materials within the starter dam appear to have been placed with lift thicknesses varying from 0.3 m to 0.6 m. Various lifts were also easily identifiable based on moisture content, silt content, and varied coloration. The relative density of the mine waste fill materials within the Lynx TDF starter dam was estimated from the LPT testing data by converting the blow counts to N_{60} values using a hammer efficiency of 85% and an LPT-SPT conversion factor of 0.6, and then correlating to relative density. The results are also plotted versus elevation for each bore hole location on the graphs included in appendix F.

The relative density of the waste rock fill materials in the starter dam, based on correlation with the equivalent N_{60} values, ranges from 90% to 100% with an average of 92%. Therefore, the Lynx TDF dam waste rock fill can be generally classified as very dense.

The geophysical site investigation program consisted of two MASW and two DST profiles. The starter dam waste rock fill shear wave average velocity (V_s) along MASW19-01 ranged from 586 m/s to 1023 m/s with an average of 710 m/s. The average velocity (V_s) at both DST locations ranges from 514 m/s to 978 m/s with an average of 746 m/s. These values of shear wave velocity are indicative of very dense soil based on Table 4.1.8.4.A in NBCC 2015.



4.3 Lynx TDF Dam – Foundation Soils

In general, the foundation soils consisted of colluvium and till-like materials, which included gravel, sand and fines. The gravel portion consist of fine to coarse grained and ranged from subrounded to angular, with the majority being subangular. Occasional cobbles were inferred from the sonic cores. The sand and fines content within the foundation soils was estimated to vary from approximately 10% to 50%. There were no iBPT or LPT testing conducted within the foundation zone. However, the geophysical results indicated that the foundation soils can be classified as very dense based on Table 4.1.8.4.A in NBCC 2015. A plot of equivalent N₆₀ values obtained from iBPT and LPT tests, and shear wave velocities obtained from MASW and DST profiles are presented in Appendix F.

Wet soil conditions were observed within the dam fill below 330.1 m elevation in BH19-01, and a sample recovered between elevation 319.3 m to 322.4 m in BH19-04 was also observed to be wet. However, no groundwater accumulations were detected in these or the other boreholes during the investigation, and the apparent wet soil conditions noted in BH19-01 and BH19-04 may have been the result of: minor perching of groundwater on less permeable soil layers, derived from downward percolation of infiltrated water or residual water following a regional groundwater table fluctuation; or water introduced to the soil by the sonic drill operation (water is used to cool the bit and some excess may have been injected into the hole). Piezometers installed in BH19-02 (30.5 m depth), BH19-03 (31 m depth) and BH19-06 (35.5 m depth) all measured negative hydrostatic head several days after installation, indicating draining of the sand packs (i.e. no groundwater present). Compression wave velocity (Vp) measured by the downhole seismic surveys in BH19-02 and BH19-03 was generally greater than 1,500 m/s (Vp of water) along the entire depth of the boreholes, and therefore cannot be used as an indication of soil saturation or the presence of a water table.

5.0 Conclusions and Limitations

The boreholes did not encounter any indications of deleterious materials contained within the starter dam as have been previously observed during foundation preparation works at the site. Both the dam shell and starter dam fill materials are interpreted to be in a generally dense state, and appear to meet the project specifications for dam embankment fill. Therefore, based on the results of this investigation and assessment, no further stability analysis or re-design of the Lynx TDF dam is warranted.

The correlations of relative density based on iBPT and LPT/SPT test results can possess a high degree of uncertainty and variability as is illustrated by the relative wide scatter of the data points in Appendix F. This variability is due mainly to the variability of the size and distribution of coarse (cobble and boulder size) fragments within the shell and starter dam material, which can significantly affect the penetration test blow counts. Therefore, the overall assessment of the material density is based in part on interpretation of the testing data, and in part on the observations of drill response, sonic core returns and Wood's previous experience with and knowledge about the site.

Deleterious materials were not observed in returns from open end iBPT, the sonic cores or the material obtained from LPT; however deleterious materials could exist in unexplored areas of the starter dam. It is Wood's opinion that if deleterious materials are present, they are not present, laterally or vertically, to an extent that would adversely impact the stability of the dam.



6.0 Closure

This report has been prepared for the exclusive use of Nyrstar Myra Falls Ltd. for specific application to the area covered within this report and is subject to the Limitations in Section 8.0.

We trust that this report meets your present requirements. If you have any questions or comments, please do not hesitate to contact Bryan Woods at 604-295-6137 or by email at bryan.woods@woodplc.com.

Yours truly,

Wood Environment & Infrastructure Solutions a Division of Wood Canada Limited

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Wood Environment & Infrastructure Solutions (2019). Letter to Keith Watson, P.Eng. Senior Tailings and Surface Engineer "Lynx TDF Starter Dam Investigation Plan", August 27, 2019.

Wood Environment & Infrastructure Solutions (2019). Letter to Keith Watson, P.Eng. Senior Tailings and Surface Engineer "Lynx TDF Starter Dam Investigation Plan Addendum – Geophysical Testing", November 7, 2019.

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The work performed in the preparation of this report and the conclusions presented herein are subject to the following:

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Advancements in the practice of geotechnical engineering, engineering geology and hydrogeology and changes in applicable regulations, standards, codes or criteria could impact the contents of the report, in which case, a supplementary report may be required. The requirements for such a review remain the sole responsibility of the Client or their agents.

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The data derived from the site investigation program and subsequent laboratory testing are interpreted by trained personnel and extrapolated across the site to form an inferred geological representation and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. Despite this investigation, conditions between and beyond the borehole/test hole locations may differ from those encountered at the borehole/test hole locations and the actual conditions at the site might differ

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from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.

Final sub-surface/bore/profile logs are developed by geotechnical engineers based upon their interpretation of field logs and laboratory evaluation of field samples. Customarily, only the final bore/profile logs are included in geotechnical engineering reports.

Bedrock, soil properties and groundwater conditions can be significantly altered by environmental remediation and/or construction activities such as the use of heavy equipment or machinery, excavation, blasting, pile-driving or draining or other activities conducted either directly on site or on adjacent terrain. These properties can also be indirectly affected by exposure to unfavorable natural events or weather conditions, including freezing, drought, precipitation and snowmelt.

During construction, excavation is frequently undertaken which exposes the actual subsurface and groundwater conditions between and beyond the test locations, which may differ from those encountered at the test locations. It is recommended that Wood be retained during construction to confirm that the subsurface conditions throughout the site do not deviate materially from those encountered at the test locations, that construction work has no negative impact on the geotechnical aspects of the design, to adjust recommendations in accordance with conditions as additional site information is gained, and to deal quickly with geotechnical considerations if they arise.

Interpretations and recommendations presented herein may not be valid if an adequate level of review or inspection by Wood is not provided during construction.

14. **Factors that may affect construction methods, costs and scheduling**: The performance of rock and soil materials during construction is greatly influenced by the means and methods of construction. Where comments are made relating to possible methods of construction, construction costs, construction techniques, sequencing, equipment or scheduling, they are intended only for the guidance of the project design professionals, and those responsible for construction monitoring. The number of test holes may not be sufficient to determine the local underground conditions between test locations that may affect construction costs, construction techniques, sequencing, equipment, scheduling, operational planning, etc.

Any contractors bidding on or undertaking the works should draw their own conclusions as to how the subsurface and groundwater conditions may affect their work, based on their own investigations and interpretations of the factual soil data, groundwater observations, and other factual information.

- 15. **Groundwater and Dewatering**: Wood will accept no responsibility for the effects of drainage and/or dewatering measures if Wood has not been specifically consulted and involved in the design and monitoring of the drainage and/or dewatering system.
- 16. **Environmental and Hazardous Materials Aspects**: Unless otherwise stated, the information contained in this report in no way reflects on the environmental aspects of this project, since this aspect is beyond the Scope of Work and the Contract. Unless expressly included in the Scope of Work, this report specifically excludes the identification or interpretation of environmental conditions such as contamination, hazardous materials, wild life conditions, rare plants or archeology conditions that may affect use or design at the site. This report specifically excludes the investigation, detection, prevention or assessment of conditions that can contribute to moisture, mould or other microbial contaminant growth and/or other moisture related deterioration, such as corrosion, decay, rot in buildings or their surroundings. Any statements in this report or on the

wood



- boring logs regarding odours, colours, and unusual or suspicious items or conditions are strictly for informational purposes
- 17. **Sample Disposal**: Wood will dispose of all uncontaminated soil and rock samples after 30 days following the release of the final geotechnical report. Should the Client request that the samples be retained for a longer time, the Client will be billed for such storage at an agreed upon rate. Contaminated samples of soil, rock or groundwater are the property of the Client, and the Client will be responsible for the proper disposal of these samples, unless previously arranged for with Wood or a third party.

Wood Environment & Infrastructure Solutions a Division of Wood Canada Limited

Wood Project # NX14001C1 | 30 April 2020

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Appendix A – iBPT Report

PRESENTATION OF SITE INVESTIGATION RESULTS

Myra Falls

Prepared for:

Wood PLC

ConeTec Job No: 19-0204607

Project Start Date: 30-Oct-2019 Project End Date: 05-Nov-2019 Report Date: 13-Nov-2019



Prepared by:

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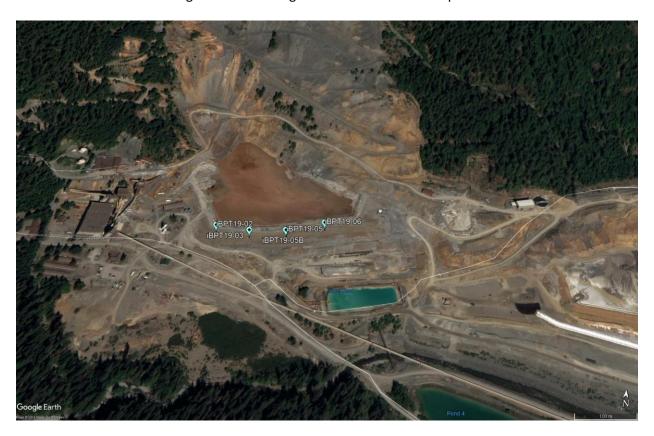
Introduction

The enclosed report presents the results of the site investigation program conducted by ConeTec Investigations Ltd. for Wood PLC at the Myra Falls mine. The program consisted of five instrumented Becker Penetration Test (iBPT) profiles.

Project Information

Project	
Client	Wood PLC
Project	Myra Falls
ConeTec project number	19-0204607

An aerial overview from Google Earth including the iBPT test locations is presented below.



Rig Description	Deployment System	Test Type
Becker hammer rig (HAV 180)	ICE 180 diesel pile driving hammer	iBPT



Coordinates		
Test Type	Collection Method	EPSG Number
iBPT	Consumer grade GPS	32610

Instrumented Becker Penetration Test (iBPT)									
Depth reference	Depths are referenced to the existing ground surface at the time of each test.								
Additional information	Tabular results are provided in Excel format files in the data release package. Plots with iBPT results and Harder & Seed (1986) results are provided in the appendices.								

Limitations

This report has been prepared for the exclusive use of Wood PLC (Client) for the project titled "Myra Falls". The report's contents may not be relied upon by any other party without the express written permission of ConeTec Investigations Ltd. (ConeTec). ConeTec has provided site investigation services, prepared the factual data reporting and provided geotechnical parameter calculations consistent with current best practices. No other warranty, expressed or implied, is made.

The information presented in the report document and the accompanying data set pertain to the specific project, site conditions and objectives described to ConeTec by the Client. In order to properly understand the factual data, assumptions and calculations, reference must be made to the documents provided and their accompanying data sets, in their entirety.



The instrumented Becker Penetration Test (iBPT) system was developed to obtain direct measurements of force and acceleration at the drill tip required to compute the energy delivered to the soil beneath the drill tip during penetration (DeJong et al. (2017)). Coupled with the measured blow counts, this provides an energy normalized blow count based on direct measurements. This direct measurement based method eliminates the long-standing challenge of estimating, and then correcting for, the shaft friction that develops along the drill string length which absorbs hammer energy.

The iBPT is an instrumentation and data acquisition system that provides automated measurements of force and acceleration at the tip and head of a drill string, in addition to measurements of bounce chamber pressure and drill string position. The system is comprised of two-foot-long instrumented pipe sections located at the head and tip of the drill string that contain redundant acceleration and strain measurements. The redundant instrumentation enables continued penetration even if one sensor malfunctions.

Data from the instrumented tip section is acquired by a data acquisition module located in the tip module while data from the head section is acquired in the iBPT trailer. The instrumented pipe sections are mechanically integrated within the conventional 6 5/8" outer diameter Becker drill system. Dynamic measurements are obtained during hammer impacts as the drill string is advanced into the ground. Continuous measurements of air pressure in the hammer bounce chamber and the string potentiometer position are also obtained during operation.

The dynamic measurements of strain and acceleration at the head and tip are processed to compute the energy delivered to each respective location. The acceleration records are integrated to obtain velocity, and then integrated a second time to obtain displacement. The force measurements account for both the dynamic force imparted during dynamic hammer blows as well as the residual locked-in force that develops at the tip due to shaft friction. Temperature correction is also applied to the tip force measurements.

Following conventional pile dynamic analysis (e.g. Rausche et al. (1972)) and the ASTM standard for energy measurement of dynamic penetrometers (D4633-16, (2016)), energy is calculated following:

$$E(kJ) = \int FVdt$$

The energy computed and reported is the residual energy as this measurement is representative of the total energy absorbed by the soil. This is different than the maximum energy delivered at the head as is conventionally used with SPT and Sy and Campanella's (S&C) Becker analysis. The iBPT energy is computed as a percentage of the theoretical hammer energy as:

$$E (\%) = \frac{\int FVdt}{11.0 \text{ kJ}}$$

The energy normalization of penetration resistance is consistent in form with SPT (ASTM D4633-16) and BPT (Sy and Campanella (1994)) methods.



The BPT measured blow counts, N_B, are then normalized to a reference hammer energy of 30% using:

$$N_{B30} = N_B \frac{E (\%)}{30}$$

The normalized to 30% hammer energy is a typical efficiency for double acting diesel hammers and is consistent with Sy and Campanella (1994).

The delivered energy ratio (DER), the ratio of the residual energy delivered to the tip of the drill string, normalized to the residual energy delivered to the head of the drill string, provides an indication of the amount of energy absorbed along the drill string due to the shaft friction. In general, the DER decreases with penetration depth and when underlying soft layers are encountered (DeJong et al. (2017)).

It is noted that DER values larger than 100% are possible as the residual energy at the tip is computed based on both the dynamic and locked-in force components. Small DER values (e.g. less than about 10%) indicate that practically all of the hammer energy (and thus penetration resistance) is being absorbed by shaft resistance along the drill string; this occurs when shaft friction is very high relative to tip resistance. This can occur in competent soils when shaft friction is high and/or in very weak soils when the soil resistance below the tip is very small. When the DER is very small (e.g. < 10%), the absolute tip measurements of force and acceleration used to compute energy can be near the performance limit, and it also produces significant reduction of $N_{\rm B}$ to compute $N_{\rm B30}$ because the ratio of E(%)/30 is very small.

When minimal energy is delivered to the tip a significant normalization factor is required to compute N_{B30} . For data reduction, a threshold limit of 2% is set for the tip residual energy, which corresponds to a normalization factor of 15. Due to the proportional form of the equation, a tip residual energy of 3% significantly reduces the normalization factor to 10 while a tip residual energy of 1% significantly increases the normalization factor to 30. Currently, application of the equation down to tip residual energies of 2% is reasonable though confidence increases rapidly with greater tip residual energy. Application of the equation to measurements with tip residual energies less than 2% extends the developed correlation beyond the database upon which it was developed.

The continuous bounce chamber pressure and pile head position measurements are used to compute hammer blows per foot (N_B) , hammer bounce chamber pressure (BCP), and displacement and depth for each hammer blow.

The correlation developed by Ghafghazi et al. (2017a) is used to estimate equivalent SPT N_{60} values as follows:

$$N_{60} = 1.8 \times N_{B30}$$

The median correlation factor of 1.8 is the best fit to the data collected and analyzed by Ghafghazi et al. (2017a), producing an estimate close to the expected 50th percentile value. This correlation value was determined based on comparisons of 364 pairs of side-by-side SPT N₆₀ and iBPT N_{B30} measurements in soils that are free of gravel (Ghafghazi et al. (2017a)). More conservative estimates of SPT N₆₀ values corresponding to a 33rd percentile value could be obtained using a correlation factor of about 1.6 or a 16th percentile value using a correlation factor of about 1.3. As detailed in Ghafghazi et al. (2017a), most of the scatter in the developed correlation is attributed to lateral spatial variability commonly present in fluvial depositional environments. As a result, and as evident in the published correlation plots, it is reasonable to observe penetration resistance differences between two adjacent soundings/borings



performed at a spacing of 10 to 20 feet, regardless of whether it is two iBPT N_{B30} soundings, two SPT N_{60} borings, or two cone penetration test (CPTu) corrected tip resistance (q_t) soundings.

Additional analysis was performed to compute equivalent SPT N_{60} values based on the Harder and Seed (1986) method. This method requires measured blows per foot (N_B), average hammer bounce chamber pressure (BCP), and correction of the measured air pressure for project elevation. The effect of shaft friction is not explicitly accounted for; instead, the accumulation of shaft friction with depth is embedded in the correction of N_B to N_{BC} even though the accumulation of shaft friction with depth depends on stratigraphy and soil type. Further discussion of the Harder and Seed method and the differences compared to the iBPT method are discussed in Ghafghazi et al. (2017b).

A summary of the iBPT soundings along with test results and individual plots are provided in the relevant appendices.

References

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The appendices listed below are included in the report:

- Instrumented Becker Penetration Test Borehole Summary and Tabular Results
- Instrumented Becker Penetration Test Plots with Energy Results and iBPT Equivalent N₆₀
- Instrumented Becker Penetration Test Plots with Harder & Seed (1986) Results



Instrumented Becker Penetration Test Borehole Summary and Tabular Results





Job No: 19-0204607 Client: Wood PLC Project: Myra Falls Start Date: 30-Oct-2019

End Date: 05-Nov-2019

	iBPT BOREHOLE SUMMARY														
Borehole ID	File Name	Date From	Date To	Predrill Depth (ft)	Start Depth (ft)	Final Depth (ft)	Northing ¹ (m)	Easting ¹ (m)	Refer To Notation						
iBPT19-02	19-0204607_iBPT02	04-Nov-2019	05-Nov-2019	0.0	0.0	81.0	5494659	311738							
iBPT19-03	19-0204607_iBPT03	03-Nov-2019	04-Nov-2019	0.0	0.0	74.9	5494655	311806							
iBPT19-05	19-0204607_iBPT05	01-Nov-2019	02-Nov-2019	0.0	0.0	75.0	5494666	311880	2						
iBPT19-05B	19-0204607_iBPT05B	02-Nov-2019	03-Nov-2019	0.0	0.0	75.3	5494666	311879							
iBPT19-06	19-0204607_iBPT06	30-Oct-2019	31-Oct-2019	0.0	0.0	82.7	5494678	311958							

^{1.} Coordinates were acquired using consumer grade GPS equipment, datum: WGS 1984 / UTM Zone 10 North.

^{2.} Drilled out to 76.75' with an open bit.



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	0.1	18.6	11	11	11	26.3	26.2	26.8	26.7	102.2	10	18	
0.1	1.0	13.2	14	14	34	25.3	23.7	24.7	24.0	101.2	27	49	
1.0	2.0	15.8	15	15	23	38.8	38.2	37.5	37.3	97.6	29	51	
2.0	3.0	17.9	21	20	27	36.8	36.3	36.9	36.8	101.4	33	60	
3.0	4.0	19.1	25	23	30	38.1	37.5	35.7	35.5	94.6	36	64	
4.0	5.0	17.6	19	18	25	37.4	36.9	34.8	34.6	93.8	29	52	
5.0	6.0	16.3	16	15	22	38.5	38.4	35.3	35.3	92.0	26	47	
6.0	7.5	15.8	15	15	22	35.4	35.1	30.2	30.0	85.3	22	40	
7.5	8.0	17.7	18	17	22	36.9	36.1	33.0	32.7	90.6	24	43	
8.0	9.0	18.9	25	23	31	35.2	33.9	30.0	29.7	87.4	31	55	
9.0	10.0	20.5	33	29	39	36.2	35.6	28.7	28.2	79.3	37	66	
10.0	11.0	20.9	35	30	40	34.5	33.5	28.1	27.7	82.7	37	66	
11.0	12.0	22.5	55	44	64	32.4	28.1	24.4	22.5	80.0	48	86	
12.0	13.0	22.5	69	52	86	30.2	23.5	21.7	19.4	82.4	55	100	
13.0	14.0	23.1	93	66	126	28.8	20.3	20.2	17.3	85.1	73	131	
14.0	15.0	23.4	109	76	152	27.2	17.5	19.9	16.9	96.8	86	154	
15.0	16.0	23.4	91	65	119	27.4	18.9	20.3	17.7	93.9	70	126	
16.0	17.0	21.4	78	57	117	25.6	17.8	19.8	17.2	96.4	67	121	
17.0	18.0	21.8	65	49	85	30.9	22.1	22.0	19.8	89.4	56	101	
18.0	19.0	22.2	77	56	104	31.9	22.4	22.7	20.8	93.0	72	130	
19.0	20.0	21.9	69	51	91	32.1	24.1	22.7	21.1	87.5	64	115	
20.0	21.0	22.0	71	53	94	31.3	23.7	22.8	21.2	89.6	67	120	
21.0	22.0	21.7	62	48	81	32.5	25.5	23.8	22.3	87.3	60	108	
22.0	23.0	20.6	46	38	58	32.7	28.0	23.9	22.8	81.6	44	79	
23.0	24.0	19.6	39	34	51	31.8	28.2	21.9	21.0	74.7	36	64	
24.0	25.0	19.5	30	27	38	34.4	31.8	21.9	21.4	67.3	27	49	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
25.0	26.0	18.5	25	23	32	34.7	34.0	22.3	22.2	65.3	24	43	
26.0	27.0	19.0	27	24	34	34.9	33.6	21.1	20.9	62.3	24	43	
27.0	27.3	18.9	30	27	39	34.1	29.0	20.9	20.2	69.7	26	47	
27.3	28.0	19.1	40	34	56	29.4	23.4	20.6	19.8	84.7	37	66	
28.0	29.0	21.3	48	39	57	33.8	26.8	23.9	22.9	85.6	44	78	
29.0	30.0	22.8	66	50	79	33.0	23.7	21.2	20.4	86.3	54	97	
30.0	31.0	23.6	96	68	126	31.0	18.1	17.5	16.2	89.4	68	122	
31.0	32.0	22.1	86	62	124	27.6	16.3	14.5	13.5	83.0	56	101	
32.0	33.0	21.1	66	50	94	26.2	18.1	12.4	11.8	65.0	37	66	
33.0	34.0	22.1	76	56	104	26.5	17.2	12.8	12.2	70.7	42	76	
34.0	35.0	20.9	51	41	65	26.6	20.0	14.7	14.2	71.1	31	55	
35.0	36.0	18.8	42	36	63	22.7	17.8	13.4	13.0	73.1	27	49	
36.0	37.0	21.2	139	94	266	22.6	11.2	11.4	9.5	84.8	84	152	
37.0	38.0	21.6	93	66	147	27.7	15.8	15.5	13.8	87.4	68	122	
38.0	39.0	22.1	92	65	136	32.0	20.1	18.7	17.8	88.2	80	145	
39.0	40.0	20.6	53	42	73	32.1	24.1	20.5	20.0	83.3	49	88	
40.0	41.0	20.6	46	38	58	32.9	25.9	19.2	18.9	73.0	36	66	
41.0	42.0	21.2	67	51	96	31.4	22.4	15.0	14.3	63.8	46	83	
42.0	43.0	20.5	63	48	95	30.0	22.3	14.3	13.7	61.6	43	78	
43.0	44.0	19.1	41	35	58	30.0	25.3	17.5	17.0	67.4	33	59	
44.0	45.0	18.4	30	27	43	30.9	27.7	18.7	18.3	66.2	26	47	
45.0	46.0	17.6	26	24	39	29.3	27.0	17.1	16.9	62.7	22	40	
46.0	47.0	19.5	38	32	49	32.9	28.7	20.2	19.8	68.9	32	58	
47.0	48.0	17.7	25	23	37	31.1	26.9	19.6	19.2	71.3	24	43	
48.0	49.0	18.0	41	34	65	31.2	24.7	18.2	17.7	71.7	38	69	
49.0	50.2	19.0	41	35	58	32.6	25.8	18.9	18.4	71.1	35	64	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
50.2	51.0	18.4	30	27	43	34.1	29.0	21.4	21.1	72.7	30	54	
51.0	52.0	19.8	48	39	69	33.0	24.9	19.5	19.1	76.6	44	79	
52.0	53.0	19.8	127	86	283	29.6	14.3	12.8	12.4	86.7	117	210	
53.0	54.0	21.0	166	109	342	30.8	15.6	14.9	14.6	93.2	166	299	
54.0	55.0	21.5	89	64	139	34.4	21.4	18.5	17.5	82.0	81	146	
55.0	56.0	20.9	75	55	116	31.6	20.7	15.6	13.8	66.8	53	96	
56.0	57.0	23.4	99	70	133	36.8	21.5	10.8	10.2	47.3	45	81	
57.0	58.0	22.4	89	64	126	35.1	21.1	9.7	9.1	43.0	38	68	
58.0	59.0	21.9	68	51	89	36.7	25.9	10.9	10.4	40.2	31	56	
59.0	60.0	21.7	127	86	222	35.7	20.1	10.7	9.1	45.4	68	122	
60.0	61.0	22.3	114	78	179	37.9	22.7	14.4	12.8	56.2	76	137	
61.0	62.0	22.9	134	91	210	38.5	21.3	16.2	14.8	69.7	104	187	
62.0	63.0	23.3	188	123	311	40.7	22.2	17.9	16.3	73.4	168	303	
63.0	64.0	22.6	108	75	163	40.5	25.4	20.4	19.2	75.5	104	188	
64.0	65.0	21.7	80	58	117	37.3	24.7	18.1	17.8	72.1	69	125	
65.0	66.0	23.6	106	74	143	40.1	23.6	17.2	16.4	69.5	78	140	
66.0	67.0	22.6	88	63	121	38.8	25.9	17.1	16.2	62.4	65	117	
67.0	68.0	20.5	53	42	73	37.0	27.8	16.0	15.6	56.3	38	68	
68.0	69.0	18.4	32	28	46	34.2	28.9	13.9	13.8	47.8	21	38	
69.0	70.0	18.5	29	26	40	34.2	30.0	11.6	11.6	38.7	15	28	
70.0	71.0	20.6	40	34	48	38.8	32.8	14.0	13.9	42.4	22	40	
71.0	72.0	20.1	40	34	49	38.8	33.4	14.4	14.2	42.4	23	42	
72.0	73.0	19.2	32	28	41	37.1	31.8	17.0	16.7	52.5	23	41	
73.0	74.0	20.9	80	58	127	39.0	26.9	17.1	15.9	59.1	67	121	
74.0	75.0	20.9	99	70	172	38.7	27.0	17.0	15.3	56.5	88	158	
75.0	76.0	21.7	85	61	128	39.5	28.0	18.4	17.5	62.5	75	134	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number	
76.0	77.0	21.9	62	47	78	40.3	30.1	20.4	20.2	67.3	53	95		
77.0	78.0	20.9	50	40	63	40.6	30.7	20.0	19.6	63.7	41	74		
78.0	79.0	21.6	85	61	130	41.0	27.9	17.9	16.8	60.1	73	131		
79.0	80.0	21.7	131	89	230	40.3	25.5	15.9	13.9	54.3	106	191		
80.0	81.0	21.5	147	98	275	39.6	27.8	18.1	15.1	54.1	138	248		

^{1.} Harder & Seed, 1986.



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	0.1	13.2	8	8	12	9.9	9.7	8.9	8.7	89.9	3	6	
0.1	1.0	14.7	13	13	21	33.7	33.1	33.5	33.2	100.3	23	42	
1.0	2.0	15.8	16	15	24	32.1	31.3	32.6	32.2	102.8	26	46	
2.0	3.0	18.5	23	22	30	34.4	33.8	35.6	35.6	105.4	36	64	
3.0	4.0	19.0	26	24	33	35.6	35.4	34.4	34.3	97.0	38	68	
4.0	5.0	18.7	27	25	36	32.0	31.4	29.4	29.1	92.7	35	63	
5.0	6.0	19.2	30	27	39	30.3	28.5	26.7	26.0	91.3	34	61	
6.0	7.0	19.4	32	28	41	29.2	27.2	25.0	24.0	88.1	33	59	
7.0	8.0	19.1	31	28	41	30.0	28.0	25.5	24.4	87.2	33	60	
8.0	9.0	19.1	30	27	39	32.1	30.8	28.0	27.3	88.6	35	64	
9.0	10.0	19.8	33	29	41	31.8	29.4	28.2	27.2	92.6	37	67	
10.0	11.0	20.1	34	30	41	31.8	29.4	28.3	27.2	92.6	37	67	
11.0	12.0	20.1	42	35	53	30.1	29.2	25.8	24.5	83.9	43	78	
12.0	13.0	20.2	41	35	51	29.7	28.3	26.1	25.0	88.2	42	76	
13.0	14.0	20.4	46	38	60	27.3	24.6	23.8	22.7	92.2	45	82	
14.0	15.0	21.2	52	42	66	26.6	21.8	22.6	21.1	96.8	46	84	
15.0	16.0	22.5	76	56	99	27.9	20.9	22.6	20.8	99.7	69	124	
16.0	17.0	20.7	68	51	104	25.8	19.5	21.2	19.5	99.7	67	121	
17.0	17.2	18.6	56	44	103	23.3	17.2	18.0	16.6	96.7	57	103	
17.2	18.0	22.7	63	48	74	33.2	26.8	24.5	23.2	86.5	57	103	
18.0	19.0	23.7	79	58	95	32.5	25.3	22.3	20.8	82.1	66	118	
19.0	20.0	22.7	75	55	95	31.6	24.6	21.5	20.1	81.8	64	115	
20.0	21.0	21.3	55	44	71	29.8	24.1	20.5	19.6	81.2	46	83	
21.0	22.0	21.3	56	44	72	30.4	24.1	23.3	22.0	91.2	53	95	
22.0	23.0	21.1	57	44	75	28.4	21.6	19.2	18.1	84.1	45	82	
23.0	24.0	20.7	58	45	82	26.5	19.3	18.2	17.1	88.6	47	84	



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
24.0	25.0	19.1	45	37	66	24.5	19.2	17.9	17.0	88.4	37	67	
25.0	26.0	18.8	41	34	59	23.7	19.7	15.8	15.2	77.3	30	54	
26.0	27.0	19.9	59	46	92	23.2	17.2	13.3	12.5	72.9	38	69	
27.0	28.0	20.8	49	40	62	28.9	22.0	14.9	14.0	63.5	29	52	
28.0	29.0	20.4	47	39	62	31.1	24.9	17.4	16.8	67.4	35	62	
29.0	30.0	23.1	103	72	146	32.1	20.1	16.5	14.5	72.5	71	127	
30.0	31.0	22.0	67	50	87	32.2	23.6	15.0	13.9	58.7	40	72	
31.0	32.0	21.3	56	44	72	31.8	24.9	17.8	16.9	67.9	41	73	
32.0	33.0	20.9	50	41	64	30.6	25.1	17.2	16.6	66.2	35	64	
33.0	34.0	21.1	63	48	87	27.7	21.0	14.5	14.0	66.8	41	73	
34.0	35.0	21.4	56	44	72	28.4	21.4	13.6	13.1	61.3	31	57	
35.0	36.0	19.9	46	38	63	25.6	20.5	10.6	10.3	50.2	22	39	
36.0	37.0	21.0	56	44	74	26.7	20.4	9.9	9.7	47.4	24	43	
37.0	38.0	20.2	37	32	45	33.1	27.7	14.1	13.8	49.9	21	37	
38.0	39.0	20.0	41	35	52	34.1	29.3	16.0	15.6	53.3	27	49	
39.0	40.0	19.9	41	35	52	33.2	29.1	16.6	16.3	55.8	28	51	
40.0	41.0	21.0	52	41	66	35.0	28.6	17.4	16.6	58.0	36	66	
41.0	42.0	18.9	37	32	52	32.3	27.5	19.2	18.4	67.1	32	57	
42.0	43.0	18.6	26	24	34	34.3	30.8	21.8	21.3	69.0	24	43	
43.0	44.0	16.2	17	17	26	31.1	30.7	18.5	18.4	59.9	16	29	
44.0	45.0	15.6	15	15	23	29.8	29.5	16.4	16.4	55.4	13	23	
45.0	46.0	16.9	19	18	27	34.2	33.2	16.2	15.8	47.7	14	26	
46.0	47.0	16.0	16	15	23	32.4	32.1	15.4	15.4	47.8	12	21	
47.0	48.0	15.8	13	13	18	32.9	32.5	16.6	16.6	50.9	10	18	
48.0	49.0	16.0	16	16	24	32.9	32.4	21.5	21.5	66.2	17	31	
49.0	50.1	15.6	15	15	23	32.7	32.3	21.3	21.3	65.9	16	29	



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
50.1	51.0	15.5	13	13	19	33.2	32.9	21.7	21.6	65.5	14	25	
51.0	52.1	14.4	11	11	17	31.0	30.6	19.5	19.3	63.1	11	20	
52.1	53.1	13.8	10	10	15	33.7	33.6	20.6	20.5	61.2	10	18	
53.1	54.0	13.6	9	9	14	31.8	31.6	18.9	18.9	59.6	9	16	
54.0	55.0	18.4	34	30	50	31.8	27.1	19.9	19.5	71.8	32	58	
55.0	56.0	18.5	34	30	49	31.2	26.3	20.0	19.5	74.2	32	57	
56.0	57.0	18.3	34	30	50	31.4	26.8	19.6	19.1	71.0	32	57	
57.0	58.0	16.6	24	22	40	31.0	28.8	18.5	18.3	63.5	24	44	
58.0	59.0	17.8	27	25	41	35.0	32.3	18.4	18.1	56.0	25	44	
59.0	60.0	16.6	20	19	31	31.7	30.3	16.2	16.0	53.0	17	30	
60.0	61.0	16.1	17	17	27	30.6	29.5	15.3	15.1	51.3	14	25	
61.0	62.0	19.8	46	38	64	35.9	30.2	18.3	17.0	56.3	36	65	
62.0	63.0	19.1	40	34	55	34.8	30.3	15.2	14.5	48.0	27	48	
63.0	64.0	17.3	23	22	35	31.2	29.0	13.2	13.1	45.1	15	27	
64.0	65.0	17.9	25	23	36	32.7	29.6	12.8	12.7	42.9	15	27	
65.0	66.0	17.6	23	22	33	33.0	30.7	9.0	9.0	29.2	10	18	
66.0	67.0	17.5	22	21	32	31.9	29.8	8.0	8.0	26.8	8	15	
67.0	68.0	16.5	15	14	19	31.4	30.2	7.9	7.9	26.0	5	9	
68.0	69.0	17.4	22	21	32	34.2	32.8	11.1	11.1	33.8	12	21	
69.0	70.0	17.5	14	14	15	34.8	34.1	11.4	11.4	33.5	6	10	
70.0	71.0	19.9	46	38	62	36.7	29.3	15.5	15.0	51.2	31	56	
71.0	72.0	20.3	57	45	84	36.1	27.4	15.9	15.2	55.5	43	77	
72.0	73.0	20.4	59	46	87	35.6	27.1	15.1	14.6	53.9	42	76	
73.0	74.0	20.7	63	48	94	35.8	27.4	13.7	13.2	48.4	42	75	
74.0	74.9	22.8	275	174	512	45.8	31.3	11.2	9.6	30.7	164	295	

^{1.} Harder & Seed, 1986.



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	0.1	18.3	10	10	10	23.3	23.1	27.2	27.1	117.1	9	16	
0.1	1.0	15.8	17	17	28	29.4	28.1	30.3	29.9	106.6	28	50	
1.0	2.0	16.7	17	16	23	38.3	37.1	37.3	36.6	98.5	28	50	
2.0	3.0	16.6	16	15	21	35.6	34.4	34.3	33.7	97.9	24	42	
3.0	4.0	17.2	17	17	23	35.3	33.9	35.8	34.7	102.5	27	48	
4.0	5.0	17.9	19	18	24	35.4	34.7	35.9	35.3	102.0	28	51	
5.0	6.0	18.6	24	22	30	32.8	32.2	29.5	29.1	90.4	29	52	
6.0	7.4	21.0	35	30	40	30.8	29.5	24.9	24.7	83.7	33	59	
7.4	8.0	18.4	31	28	45	27.6	26.4	23.5	22.8	86.4	34	61	
8.0	9.0	19.6	29	26	36	33.9	31.7	33.4	32.6	102.8	39	70	
9.0	10.0	20.9	25	23	27	30.9	28.4	27.8	26.8	94.5	24	43	
10.0	11.0	21.6	27	25	27	30.8	27.9	25.8	24.8	89.1	22	40	
11.0	12.5	21.3	31	28	35	30.6	27.8	24.9	23.9	86.1	28	50	
12.5	13.1	21.7	21	20	21	28.1	24.6	19.0	18.0	73.2	13	23	
13.1	14.1	21.2	21	20	21	29.6	27.5	21.5	20.5	74.8	14	26	
14.1	15.0	20.5	33	29	39	29.2	27.1	23.2	22.3	82.5	29	52	
15.0	16.2	20.4	36	31	43	26.4	23.7	20.4	19.4	81.6	28	50	
16.2	17.1	21.3	3	3	3	24.3	18.4	16.4	14.6	79.4	1	3	
17.1	19.2	19.9	9	9	9	27.1	19.6	18.2	17.0	86.8	5	9	
19.2	24.4	19.7	1	1	1								2
24.4	25.0	19.0	24	22	29	30.0	27.2	22.0	20.9	76.9	20	36	
25.0	26.0	17.5	25	23	38	27.9	26.5	19.8	19.4	73.1	25	44	
26.0	27.3	18.2	22	20	28	28.8	27.1	19.2	19.0	70.1	18	32	
27.3	28.0	18.4	34	30	49	31.0	27.2	19.7	18.9	69.5	31	56	
28.0	29.0	19.6	30	27	37	35.8	32.0	23.8	23.2	72.5	29	52	
29.0	30.0	20.1	32	29	39	35.7	31.7	23.0	22.4	70.5	29	52	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
30.0	31.0	19.5	34	30	43	33.4	29.5	19.4	18.9	64.1	27	49	
31.0	32.1	21.3	4	4	4								2
32.1	33.1	20.5	13	13	13	32.4	25.8	17.6	16.9	65.5	7	13	
33.1	34.0	19.9	25	23	29	32.1	26.6	18.2	17.4	65.5	17	30	
34.0	35.0	19.7	21	20	23	37.9	36.3	21.1	20.7	57.0	16	29	
35.0	36.1	19.0	26	24	32	34.3	31.5	17.7	17.4	55.3	19	33	
36.1	37.4	20.5	5	5	5	28.0	21.5	11.3	10.8	50.2	2	3	
37.4	38.1	20.6	29	26	34	37.2	26.8	18.4	17.8	66.5	20	36	
38.1	39.0	20.4	10	10	10	36.7	28.0	19.1	18.2	64.8	6	11	
39.0	40.0	19.2	37	32	49	34.0	28.3	20.8	19.8	70.0	32	58	
40.0	41.0	18.8	27	25	35	34.9	31.8	21.6	21.1	66.5	25	44	
41.0	42.0	19.2	23	21	27	37.4	35.3	20.2	19.9	56.3	18	32	
42.0	43.0	18.4	22	20	27	37.4	36.6	18.6	18.4	50.4	17	30	
43.0	44.0	16.6	16	16	22	34.1	33.6	14.7	14.6	43.5	11	19	
44.0	45.0	17.0	17	17	23	34.7	34.3	15.6	15.5	45.2	12	21	
45.0	46.0	17.4	19	18	25	36.7	36.7	15.7	15.6	42.6	13	23	
46.0	47.0	17.3	19	18	26	36.6	36.6	17.6	17.4	47.6	15	27	
47.0	48.0	18.2	16	16	18	38.1	37.9	18.1	18.0	47.5	11	19	
48.0	49.0	18.3	19	18	22	41.1	40.6	23.3	23.1	56.8	17	30	
49.0	50.0	15.5	13	13	19	34.6	34.1	22.2	22.1	64.8	14	25	
50.0	51.0	13.1	10	10	17	28.2	28.1	16.5	16.4	58.6	9	17	
51.0	52.0	15.2	12	12	16	34.4	34.3	20.8	20.7	60.4	11	20	
52.0	53.1	15.3	12	12	17	37.7	37.7	23.6	23.6	62.7	13	24	
53.1	54.0	15.5	12	12	17	36.0	35.9	23.0	23.0	64.0	13	23	
54.0	55.0	15.9	14	13	19	34.4	33.8	22.1	21.9	64.8	14	25	
55.0	56.0	16.8	19	18	27	33.9	32.6	21.8	21.6	66.2	19	35	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
56.0	57.1	19.7	38	32	48	35.6	26.6	19.9	19.7	73.8	31	57	
57.1	58.0	20.4	47	39	62	36.5	26.1	21.5	21.4	82.1	44	80	
58.0	59.0	20.2	57	45	85	36.0	26.4	19.9	19.8	75.0	56	101	
59.0	60.0	20.1	49	40	68	35.1	26.7	16.8	16.4	61.3	37	67	
60.0	61.0	19.3	52	42	82	34.8	28.1	17.7	16.8	59.7	46	82	
61.0	62.0	17.6	25	23	38	34.0	31.2	19.0	18.4	58.9	23	42	
62.0	63.0	17.5	24	22	35	32.1	30.0	19.5	19.0	63.4	22	40	
63.0	64.0	18.9	34	30	46	35.0	30.8	17.7	17.3	56.3	27	48	
64.0	65.2	19.9	41	34	52	37.3	31.2	16.7	16.4	52.4	28	51	
65.2	66.0	20.3	24	22	26	38.5	33.0	18.8	18.5	56.2	16	29	
66.0	67.0	19.9	27	25	32	37.7	33.7	14.4	14.2	42.3	15	27	
67.0	68.0	18.5	26	24	34	34.7	32.3	9.0	9.0	27.9	10	18	
68.0	69.0	17.7	27	24	40	33.0	30.7	7.6	7.6	24.6	10	18	
69.0	70.0	18.2	23	22	31	35.1	33.5	5.9	5.8	17.4	6	11	
70.0	71.0	18.4	22	20	27	35.6	33.8	3.8	3.8	11.1	3	6	
71.0	72.0	17.0	22	20	33	32.2	30.6	3.2	3.2	10.4	4	6	
72.0	73.0	17.2	19	19	27	33.5	32.3	3.9	3.9	12.0	3	6	
73.0	74.0	17.4	20	19	28	33.0	31.3	7.8	7.7	24.4	7	13	
74.0	75.0	19.1	43	36	63	37.0	30.9	12.2	11.6	37.5	24	44	

^{1.} Harder & Seed, 1986.

^{2.} Energy results are not presented due to equipment issues.



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	0.1	19.6	11	11	11	12.2	12.2	16.8	16.7	137.8	6	11	
0.1	1.0	16.7	17	16	23	34.6	33.0	34.1	33.2	100.5	25	46	
1.0	2.0	18.1	21	20	27	35.2	34.3	35.7	35.4	103.3	32	57	
2.0	3.0	17.2	18	18	25	34.8	33.5	33.0	32.3	96.4	27	48	
3.0	4.0	18.3	24	22	31	34.0	33.4	32.6	32.6	97.5	34	61	
4.0	5.0	19.5	30	27	38	32.3	31.9	29.8	29.7	93.0	38	68	
5.0	6.0	20.0	36	31	44	29.9	28.6	26.2	25.9	90.6	38	68	
6.0	7.3	19.4	29	26	36	30.2	29.4	25.3	25.0	85.1	30	54	
7.3	8.0	18.9	30	27	40	31.0	29.8	26.4	26.0	87.3	35	62	
8.0	9.0	18.4	27	25	37	31.2	29.2	26.8	26.6	90.9	33	59	
9.0	10.0	20.6	45	37	56	29.5	26.8	24.2	23.0	86.0	43	77	
10.0	11.0	19.9	45	37	61	27.9	25.1	22.1	21.0	83.4	43	77	
11.0	12.0	19.7	45	37	62	27.2	24.7	20.4	19.3	78.4	40	72	
12.0	13.0	20.0	46	38	61	28.3	25.3	19.6	18.6	73.5	38	68	
13.0	14.0	19.5	40	34	53	29.3	26.1	21.3	20.2	77.5	36	64	
14.0	15.0	19.5	40	34	52	28.3	25.4	20.5	19.5	76.7	34	61	
15.0	16.0	22.2	62	48	77	29.8	24.2	20.7	19.2	79.6	49	89	
16.0	17.0	23.2	84	60	107	31.1	21.7	21.3	19.3	89.3	69	124	
17.0	18.0	23.1	66	50	77	36.9	26.6	23.7	22.1	83.2	57	102	
18.0	19.0	23.8	87	62	107	36.0	25.4	22.1	20.3	79.6	72	130	
19.0	20.0	22.5	69	52	86	34.0	26.4	21.4	19.8	74.9	57	102	
20.0	21.0	22.1	65	49	83	33.3	24.9	18.6	17.4	69.9	48	87	
21.0	22.0	22.4	67	50	83	34.4	25.9	18.7	17.3	66.8	48	86	
22.0	23.0	23.1	69	52	83	34.0	25.9	18.0	16.6	64.1	46	83	
23.0	24.0	22.7	56	44	64	35.4	27.6	20.6	19.3	69.8	41	74	
24.0	25.0	20.3	38	33	46	32.9	27.4	21.4	20.4	74.4	31	56	



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
25.0	26.0	20.2	40	34	50	32.5	26.3	21.1	20.0	76.1	33	60	
26.0	27.0	20.5	39	33	46	35.0	30.0	22.6	21.6	71.9	33	59	
27.0	28.0	20.8	31	28	36	35.4	31.6	26.2	23.9	75.6	29	52	
28.0	29.0	20.6	29	26	34	36.2	32.9	26.8	26.2	79.7	30	53	
29.0	30.0	20.7	36	31	42	32.8	28.7	22.1	21.5	74.9	30	54	
30.0	31.0	20.6	32	29	38	32.8	29.3	20.9	20.5	70.0	26	47	
31.0	32.0	23.6	69	52	79	33.2	24.7	18.4	17.6	71.2	46	83	
32.0	33.0	23.2	63	48	72	32.0	24.8	17.4	16.7	67.3	40	72	
33.0	34.0	21.2	41	35	48	30.9	26.3	15.9	15.6	59.6	25	45	
34.0	35.0	21.9	42	35	47	31.1	26.2	13.0	12.9	49.2	20	36	
35.0	36.0	22.0	47	39	54	30.5	23.9	12.5	12.3	51.5	22	40	
36.0	37.0	21.2	58	45	77	27.7	20.5	11.7	11.4	55.7	29	53	
37.0	38.0	20.8	43	36	52	32.7	26.7	15.9	15.3	57.3	26	48	
38.0	39.0	21.1	41	34	47	34.7	29.6	18.6	18.1	61.0	28	51	
39.0	40.0	21.0	44	37	53	34.7	28.7	19.5	18.6	64.9	33	59	
40.0	41.0	19.2	29	26	36	33.9	30.7	20.0	19.5	63.4	23	42	
41.0	42.0	18.0	19	18	24	33.1	32.1	19.5	19.4	60.3	15	28	
42.0	43.0	18.9	24	23	30	33.8	32.3	20.6	20.2	62.7	20	36	
43.0	44.0	17.6	20	19	27	33.0	32.0	19.9	19.6	61.1	18	32	
44.0	45.0	17.7	24	23	35	28.8	25.5	17.0	16.7	65.5	19	35	
45.0	46.0	20.3	47	38	61	30.8	22.8	17.8	17.1	74.9	35	62	
46.0	47.0	20.2	52	42	74	28.4	20.7	15.5	14.8	71.3	36	66	
47.0	48.0	20.2	55	43	79	31.8	23.1	14.2	13.8	59.7	36	65	
48.0	49.0	20.7	59	46	84	35.1	26.3	16.5	16.0	61.0	45	81	
49.0	50.0	19.6	52	41	78	31.5	24.2	15.3	14.8	61.3	39	69	
50.0	51.0	18.3	32	28	46	30.4	26.2	15.9	15.3	58.5	23	42	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
51.0	52.0	17.9	28	26	42	29.1	25.5	15.7	15.4	60.2	21	39	
52.0	53.0	18.7	31	27	42	30.9	26.9	16.0	15.6	58.1	22	39	
53.0	54.0	17.6	29	26	46	27.1	23.4	13.0	12.7	54.3	19	35	
54.0	55.0	16.2	27	25	51	23.3	20.1	11.1	10.8	53.5	18	33	
55.0	56.0	16.4	36	31	70	21.6	17.4	9.9	9.4	54.0	22	39	
56.0	57.0	19.2	63	48	111	28.4	20.2	12.8	12.0	59.7	45	80	
57.0	58.0	18.6	35	30	50	32.6	27.1	17.1	16.6	61.1	28	50	
58.0	59.0	15.9	22	21	40	29.5	27.1	17.3	16.8	62.2	22	40	
59.0	60.0	14.5	15	14	27	27.1	26.4	14.7	14.5	54.9	13	23	
60.0	61.0	14.5	14	13	24	27.8	27.0	13.0	12.9	47.9	10	19	
61.0	62.0	15.6	20	19	38	28.1	25.7	14.5	14.2	55.2	18	32	
62.0	63.0	15.9	22	20	39	27.9	25.1	15.5	15.2	60.8	20	36	
63.0	64.0	16.7	25	23	43	29.3	26.4	15.6	15.4	58.1	22	40	
64.0	65.0	18.4	28	25	39	33.5	30.4	14.5	14.2	46.8	19	33	
65.0	66.0	20.0	44	37	58	34.8	28.8	13.4	13.2	45.7	25	46	
66.0	67.0	20.4	59	46	88	34.3	26.1	12.9	12.3	47.1	36	65	
67.0	68.0	19.5	49	39	72	34.0	27.3	12.4	11.9	43.6	29	51	
68.0	69.0	19.5	38	32	49	36.0	32.0	9.2	8.8	27.5	14	26	
69.0	70.0	17.9	35	30	55	29.7	26.2	4.5	4.3	16.3	8	14	
70.0	71.0	18.8	39	33	55	30.9	27.0	3.6	3.5	12.8	6	11	
71.0	72.0	19.8	44	37	59	33.7	28.5	4.7	4.6	16.0	9	16	
72.0	73.0	18.9	38	33	53	30.3	26.0	3.9	3.8	14.6	7	12	
73.0	74.0	22.2	114	78	182	37.0	23.2	7.8	6.8	29.4	41	74	
74.0	75.0	22.0	169	112	311	37.9	23.6	8.8	7.7	32.7	80	144	
75.0	75.3	22.3	500	308	1018	41.0	23.7	9.3	8.5	36.1	290	521	

^{1.} Harder & Seed, 1986.



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
0.0	0.1	10.2	6	6	10	11.2	11.2	8.6	8.5	76.4	3	5	
0.1	1.0	16.0	16	15	23	34.0	33.3	33.9	33.8	101.5	26	47	
1.0	2.0	16.3	17	16	24	35.8	34.3	35.0	34.2	99.6	27	49	
2.0	3.0	18.4	22	20	27	35.1	33.9	36.4	36.3	107.3	33	59	
3.0	4.0	19.4	27	25	33	36.6	35.8	37.0	37.0	103.4	41	73	
4.0	5.0	20.6	33	29	39	32.9	31.8	29.8	29.4	92.3	38	69	
5.0	6.0	22.6	50	40	56	31.0	28.4	25.9	25.3	89.1	47	85	
6.0	7.0	23.3	57	44	63	31.1	27.3	25.9	25.5	93.4	54	96	
7.0	8.0	22.3	51	41	58	31.2	26.0	26.1	25.0	96.3	48	87	
8.0	9.0	21.6	57	45	72	29.2	24.0	23.2	21.8	91.0	52	94	
9.0	10.0	20.9	52	41	67	29.2	23.3	23.8	22.2	95.3	50	89	
10.0	11.0	20.7	48	39	61	29.1	25.1	24.3	23.2	92.3	47	85	
11.0	12.0	21.4	52	42	64	30.5	25.8	23.4	22.2	86.1	47	85	
12.0	13.0	21.6	51	41	61	31.3	26.9	23.1	21.8	81.1	44	80	
13.0	14.0	22.3	58	45	69	31.7	26.5	21.3	20.0	75.6	46	83	
14.0	15.0	23.3	71	53	84	32.0	23.8	21.6	19.6	82.2	55	99	
15.0	16.0	24.0	74	55	86	33.1	24.0	24.6	22.1	92.3	63	114	
16.0	17.0	22.3	66	50	83	30.5	23.7	22.5	20.5	86.4	57	102	
17.0	18.0	21.3	44	36	51	37.2	31.4	28.5	27.2	86.6	46	83	
18.0	19.0	21.9	42	35	47	36.7	31.3	26.9	26.1	83.5	41	74	
19.0	20.0	24.4	76	56	87	37.1	26.0	24.7	23.2	89.2	67	121	
20.0	21.0	23.1	65	49	75	35.4	25.3	24.0	22.4	88.7	56	101	
21.0	22.0	21.8	56	44	68	33.5	26.1	21.2	20.2	77.4	46	82	
22.0	23.0	21.7	51	41	60	33.6	26.5	23.5	22.4	84.6	45	81	
23.0	24.0	19.1	30	27	39	33.3	30.6	22.8	22.3	72.8	29	52	
24.0	25.0	18.4	27	24	36	32.1	29.8	23.6	23.3	78.1	28	50	



INSTRUMENTED BECKER PENETRATION TEST RESULTS													
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
25.0	26.0	20.1	35	30	42	33.0	28.7	22.2	21.7	75.6	30	55	
26.0	27.0	21.0	44	37	53	32.6	27.0	23.7	22.7	84.1	40	72	
27.0	28.0	22.2	44	37	50	38.4	29.9	27.7	26.0	86.8	43	78	
28.0	29.0	23.1	52	42	58	39.3	30.8	23.3	22.4	72.8	43	78	
29.0	30.0	22.8	55	43	62	37.1	29.2	23.3	22.5	77.0	46	84	
30.0	31.0	22.2	58	45	69	35.6	27.4	23.8	22.7	82.9	52	94	
31.0	32.0	24.2	89	63	107	36.9	24.4	21.7	20.4	83.6	73	131	
32.0	33.0	23.7	82	60	100	37.3	27.0	21.0	19.6	72.7	65	118	
33.0	34.0	22.5	59	46	69	41.9	36.8	20.9	19.8	53.9	46	82	
34.0	35.0	23.0	52	42	58	38.0	30.3	19.4	18.8	62.0	36	65	
35.0	36.0	23.1	50	41	56	38.2	31.2	20.2	19.7	63.0	37	66	
36.0	37.0	23.5	59	46	65	36.9	29.8	17.6	17.0	57.1	37	66	
37.0	37.1	22.1	58	45	70	36.2	27.7	15.7	15.0	54.3	35	63	
37.1	38.0	22.4	74	55	97	32.9	20.9	14.5	14.3	68.4	46	83	
38.0	39.0	22.8	63	48	74	35.3	25.9	17.5	17.3	66.9	43	77	
39.0	40.0	21.2	46	38	55	34.6	28.7	24.2	23.9	83.4	44	79	
40.0	41.0	20.5	38	33	45	34.1	29.8	20.8	20.5	68.9	31	55	
41.0	42.0	20.2	34	30	41	33.2	29.4	16.6	16.5	56.0	23	41	
42.0	43.0	20.4	38	33	46	32.6	28.0	17.2	17.0	60.9	26	47	
43.0	44.0	20.9	47	38	58	31.1	25.6	17.5	17.2	67.1	33	60	
44.0	45.0	20.7	37	32	44	31.9	28.0	18.8	18.5	66.2	27	49	
45.0	46.0	18.8	30	27	41	27.3	24.4	14.6	14.4	59.1	20	36	
46.0	47.2	17.3	20	19	27	26.5	25.6	12.9	12.8	49.9	12	21	
47.2	48.0	17.2	14	14	17	33.2	32.2	14.1	14.1	43.8	8	14	
48.0	49.0	15.9	12	12	15	35.6	35.4	19.1	18.9	53.4	9	17	
49.0	50.1	15.9	13	13	17	34.5	34.5	19.2	19.1	55.6	11	20	



	INSTRUMENTED BECKER PENETRATION TEST RESULTS												
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
50.1	51.0	16.8	14	14	18	36.3	35.9	25.2	25.1	69.9	15	27	
51.0	52.0	15.7	12	12	16	36.0	35.6	26.8	26.5	74.6	14	25	
52.0	53.0	15.6	12	12	15	35.7	35.4	28.7	28.5	80.6	14	26	
53.0	54.0	15.6	12	12	16	33.6	33.2	22.1	21.9	65.9	12	21	
54.0	55.0	16.1	13	13	16	32.5	32.1	18.5	18.4	57.4	10	18	
55.0	56.0	15.1	12	12	18	30.4	30.2	17.0	16.9	56.0	10	18	
56.0	57.2	14.8	11	11	16	28.6	28.3	19.3	19.2	67.7	10	18	
57.2	58.0	16.9	15	15	19	34.6	33.8	26.5	26.2	77.3	17	30	
58.0	59.0	20.1	49	40	67	34.4	26.4	22.8	22.7	86.1	51	91	
59.0	60.0	22.9	151	101	246	35.4	20.1	16.6	16.4	81.8	135	242	
60.0	61.0	22.7	173	114	295	37.2	22.0	19.2	18.7	84.9	184	331	
61.0	62.0	20.7	106	74	196	32.2	21.4	17.9	17.2	80.6	113	203	
62.0	63.0	20.7	68	51	105	33.1	23.2	17.8	17.3	74.5	60	109	
63.0	64.0	19.2	47	39	72	31.1	25.0	18.3	17.7	71.0	43	77	
64.0	65.0	17.5	27	25	42	27.5	25.2	16.2	15.8	62.8	22	40	
65.0	66.0	16.6	19	18	29	27.2	25.6	9.8	9.7	38.0	9	17	
66.0	67.0	16.7	19	18	29	28.9	27.3	9.8	9.8	36.0	9	17	
67.0	68.0	16.5	11	11	11	34.1	33.7	17.0	17.0	50.3	6	11	
68.0	69.1	15.4	12	12	16	33.9	33.7	15.1	15.1	44.8	8	14	
69.1	70.1	15.3	12	12	16	33.0	32.6	14.4	14.4	44.2	8	14	
70.1	71.0	16.3	14	14	19	33.3	32.2	12.4	12.4	38.4	8	14	
71.0	72.1	16.1	13	13	17	34.2	33.8	12.4	12.3	36.5	7	13	
72.1	73.1	15.6	12	12	15	33.5	33.1	9.5	9.5	28.7	5	9	
73.1	74.0	15.8	12	12	15	33.1	32.7	9.8	9.8	29.9	5	9	
74.0	75.0	15.6	13	12	17	32.9	32.6	9.6	9.5	29.0	5	10	
75.0	76.0	15.5	13	13	19	29.6	29.0	8.5	8.5	29.4	5	10	

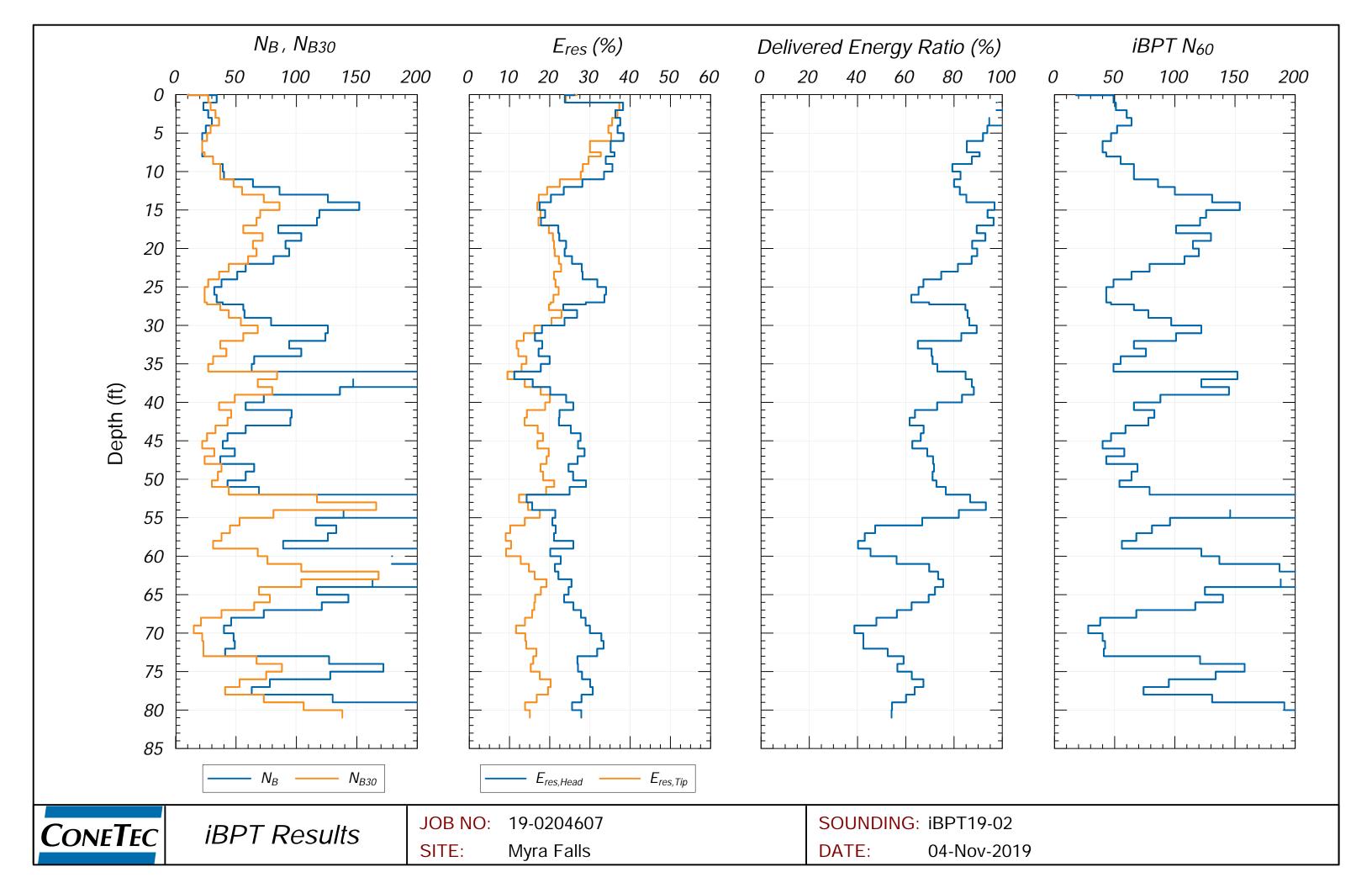


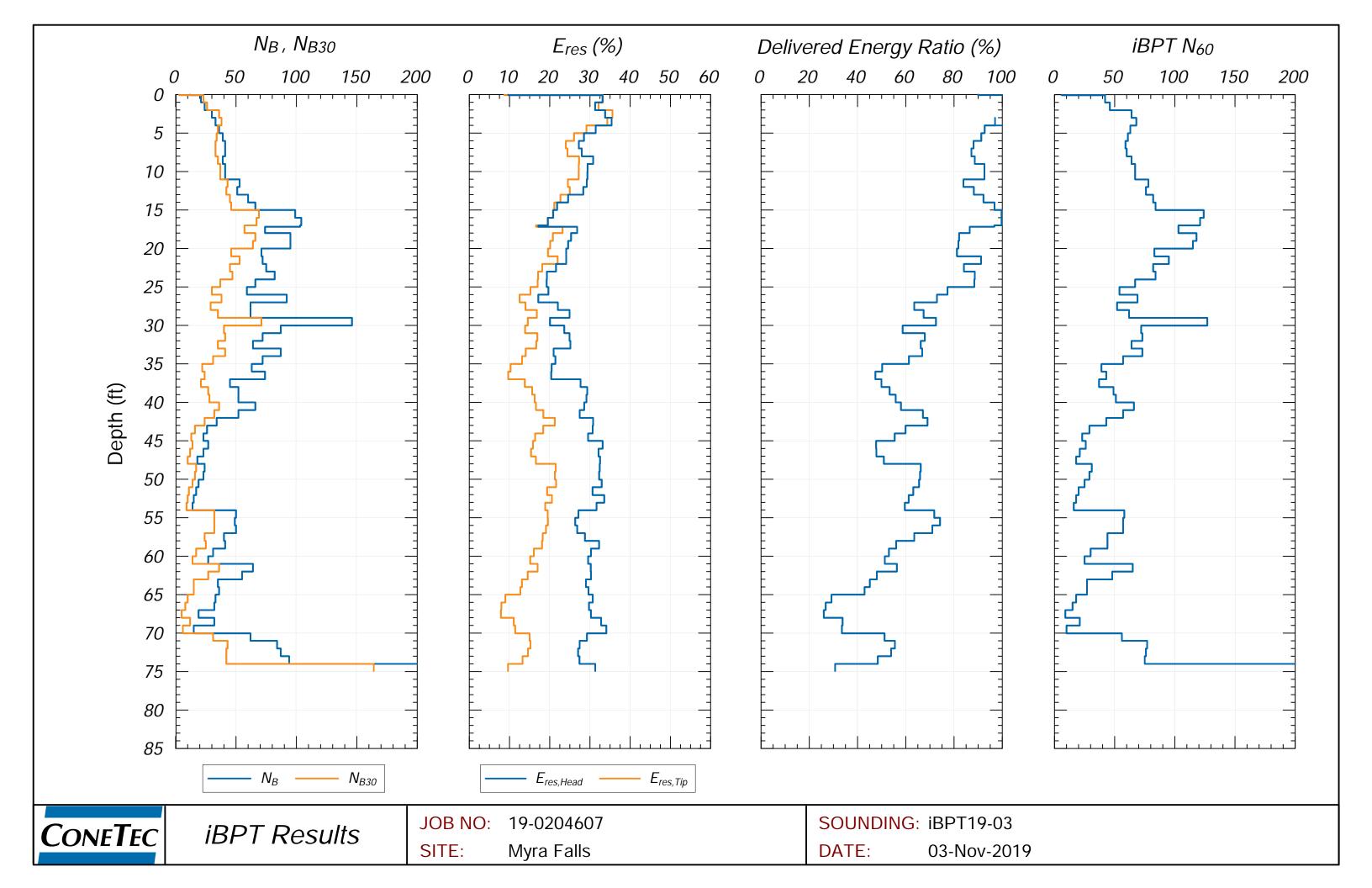
			ı	INSTRU	MENTED BEG	CKER PENET	RATION TEST	RESULTS					
Start Depth (ft)	End Depth (ft)	Average Bounce Chamber Pressure (BCP) (psi)	N _{BC} ¹	N ₆₀ ¹	N _B (blows per foot)	Average Head Max Energy E _{max,head} (%)	Average Head Residual Energy E _{res,head} (%)	Average Tip Max Energy E _{max,tip} (%)	Average Tip Residual Energy E _{res,tip} (%)	Delivered Energy Ratio (DER)	N _{B30}	iBPT N ₆₀	Refer to Notation Number
76.0	77.3	15.3	13	13	19	28.7	28.3	8.2	8.2	28.9	5	9	
77.3	78.1	15.9	8	8	8	34.6	34.5	10.9	10.9	31.5	3	5	
78.1	79.0	15.0	12	12	17	29.8	29.4	10.9	10.9	37.0	6	11	
79.0	80.0	18.8	40	34	57	33.4	28.5	18.3	18.0	62.9	34	61	
80.0	81.0	18.4	44	36	69	31.4	25.8	19.4	18.7	72.4	43	77	
81.0	82.0	19.3	133	90	328	31.1	20.1	14.0	12.3	61.3	134	242	
82.0	82.7	20.9	175	115	371	34.4	19.0	12.0	9.4	49.5	116	210	

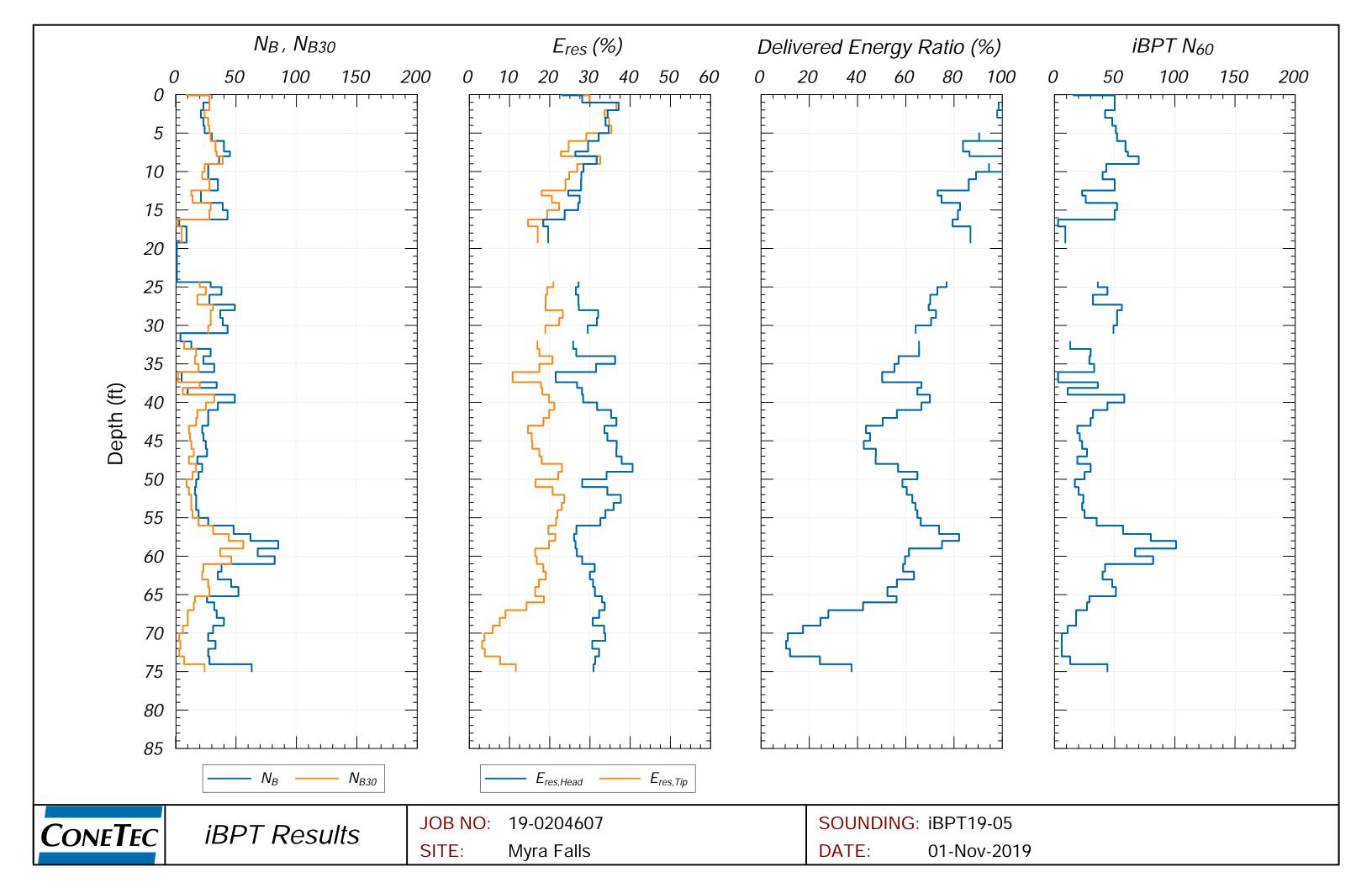
^{1.} Harder & Seed, 1986.

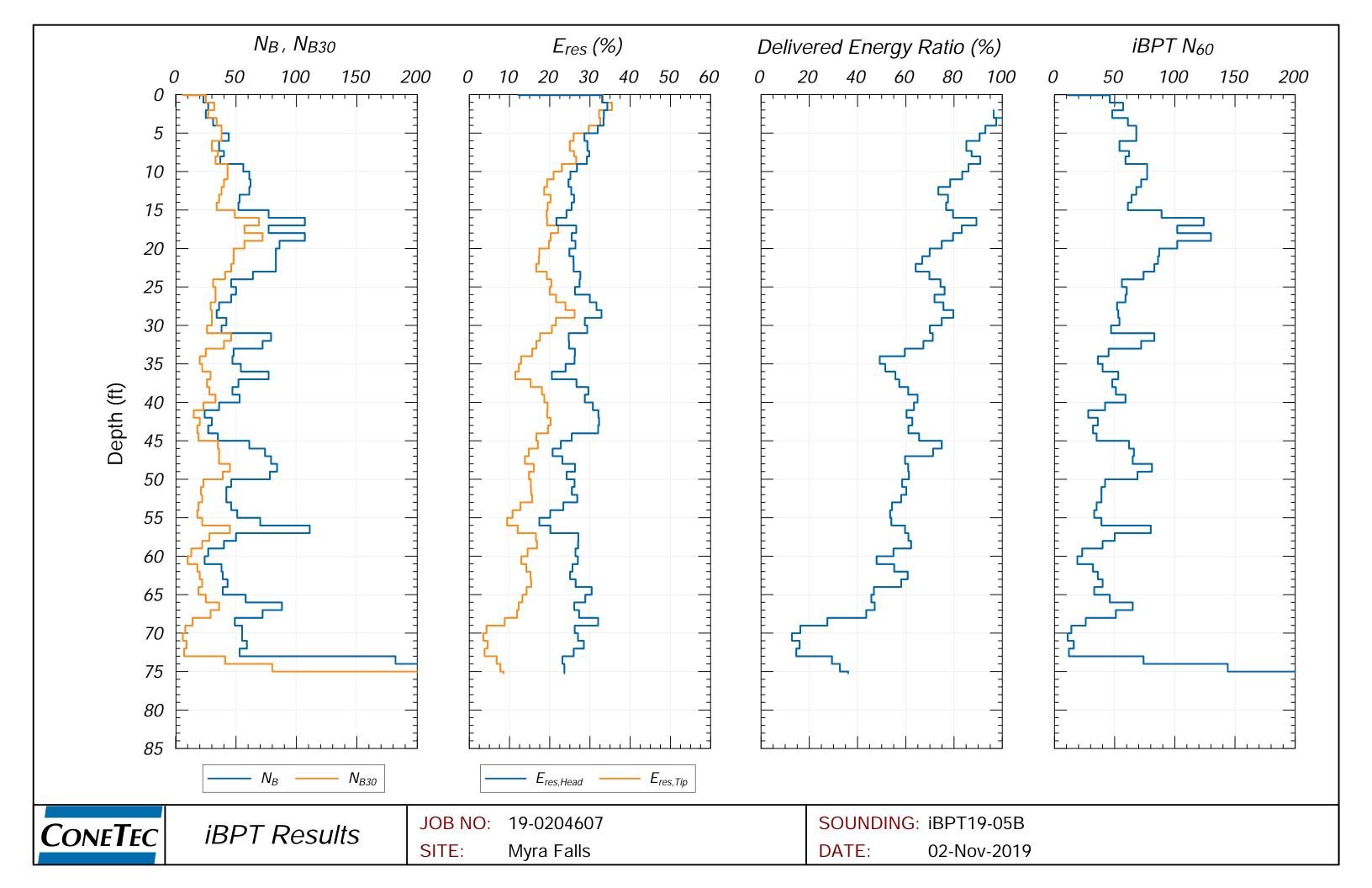
Instrumented Becker Penetration Test Plots with Energy Results and iBPT Equivalent N_{60}

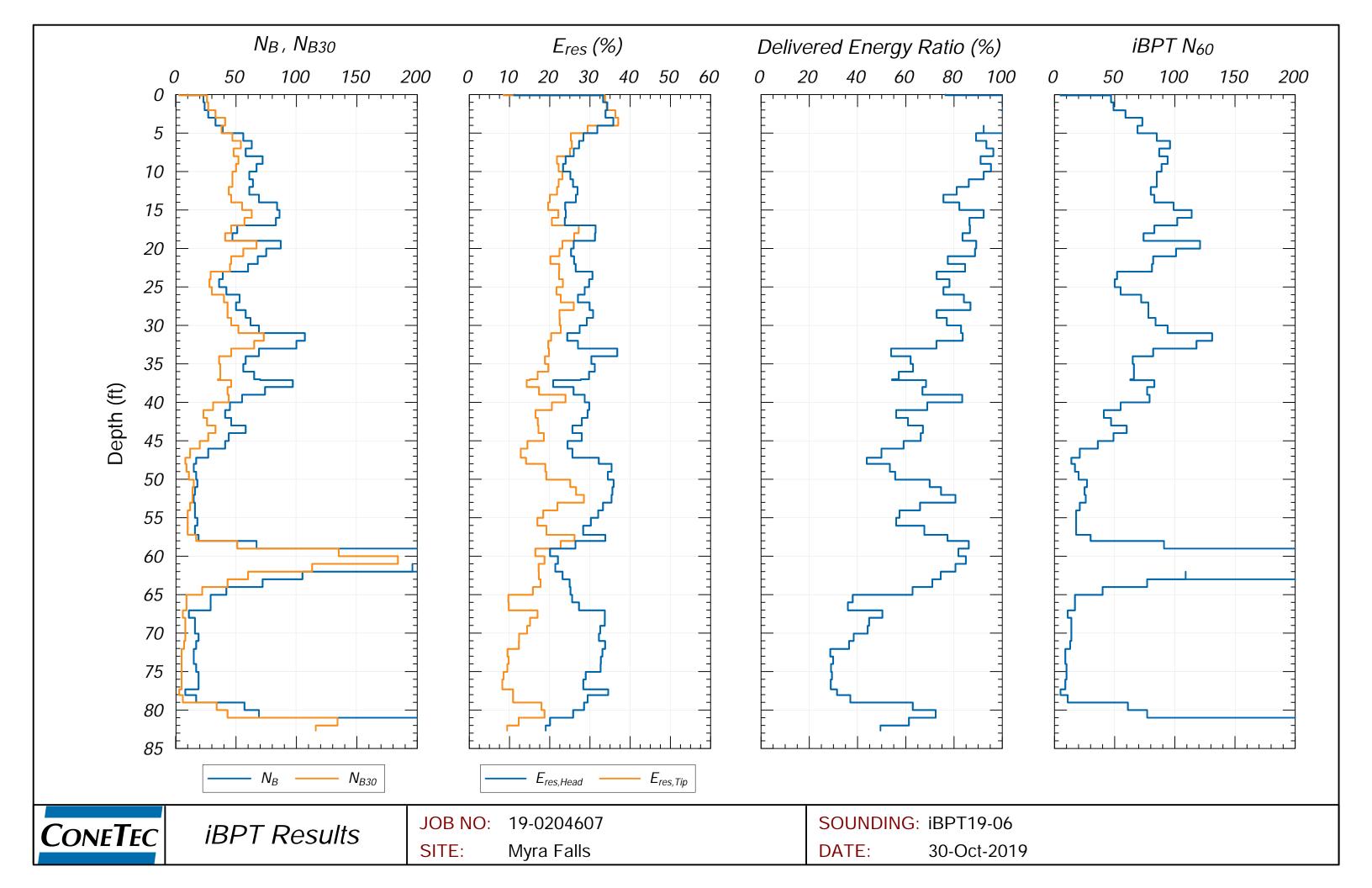






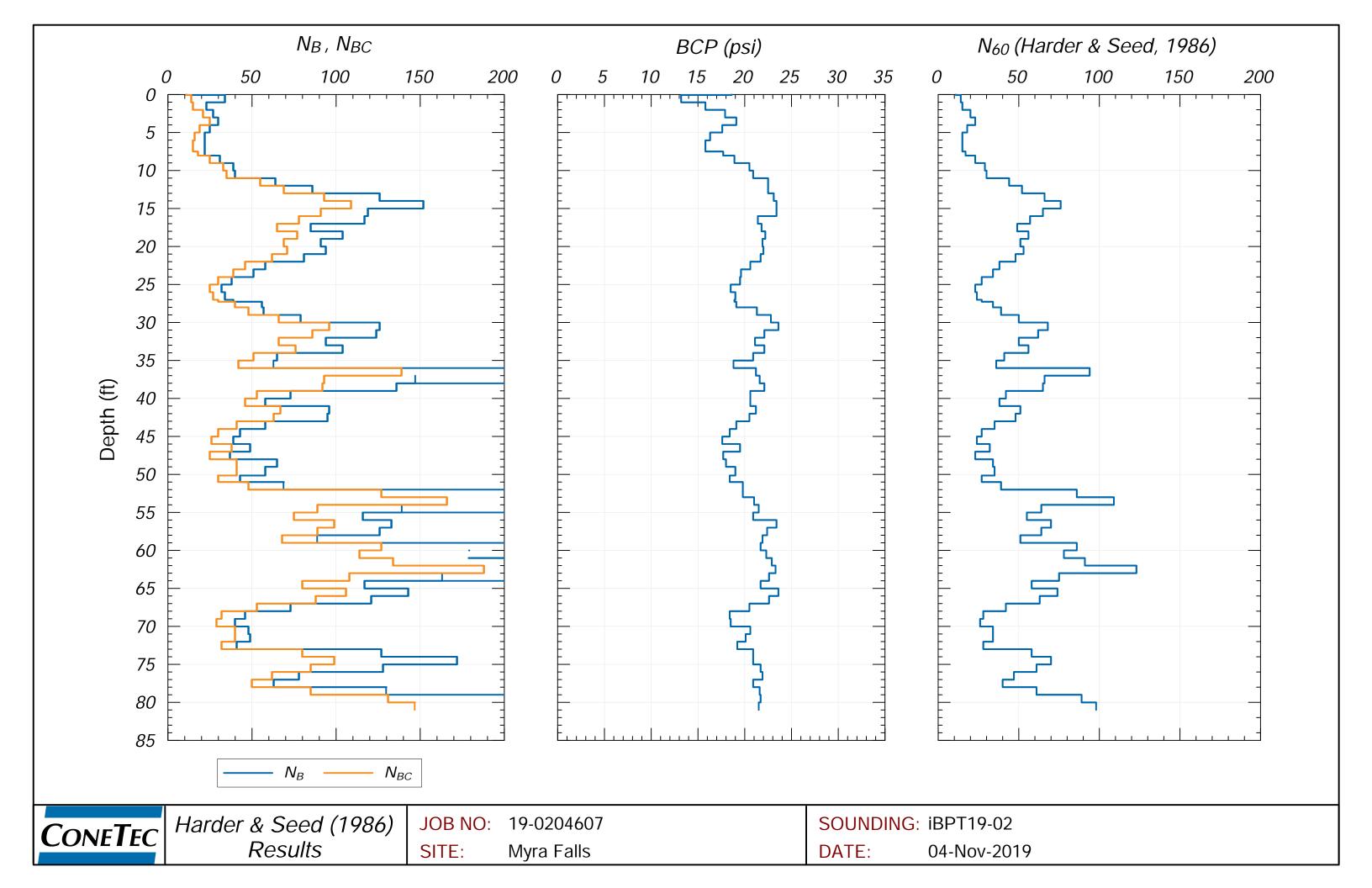


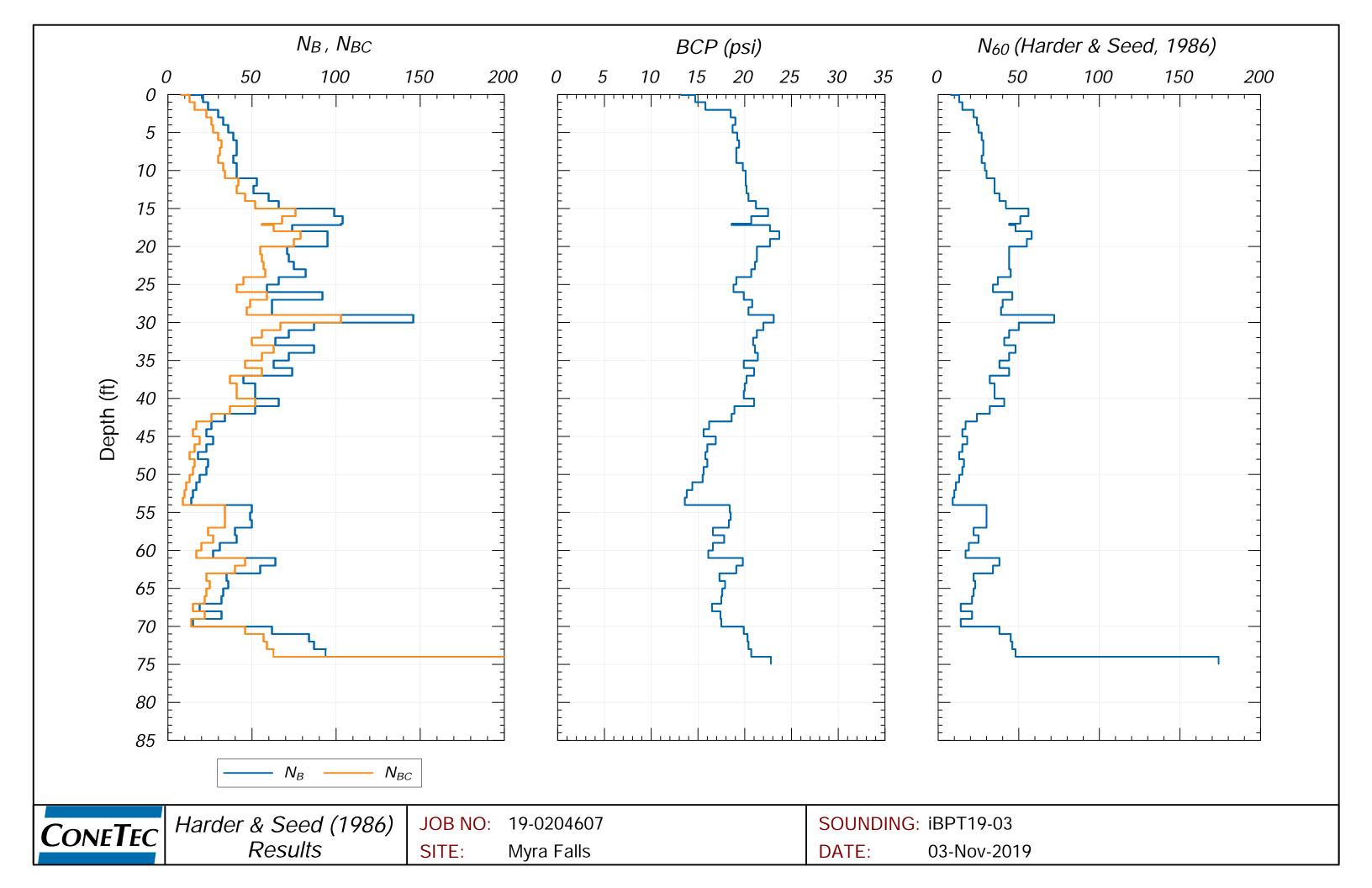


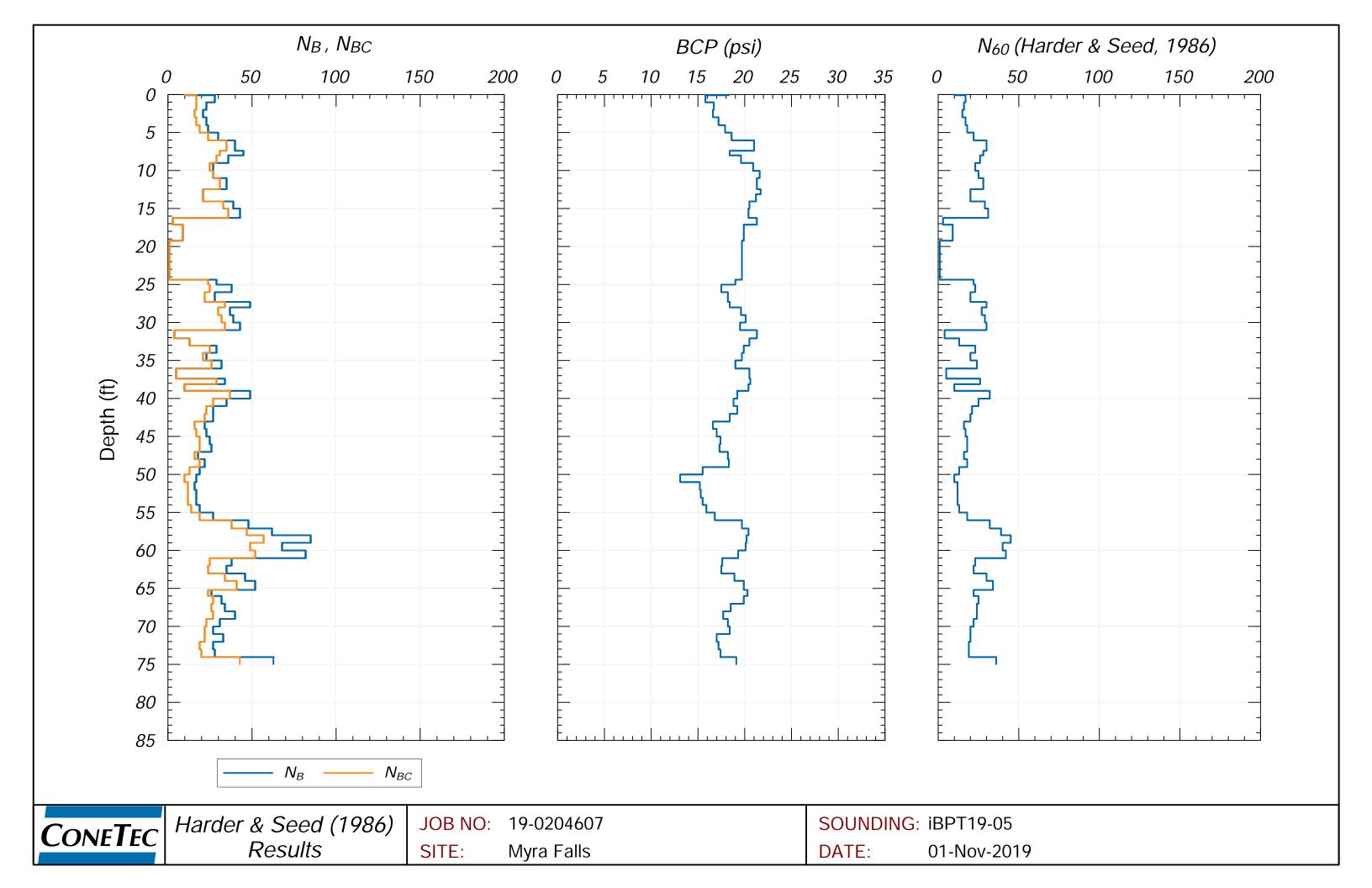


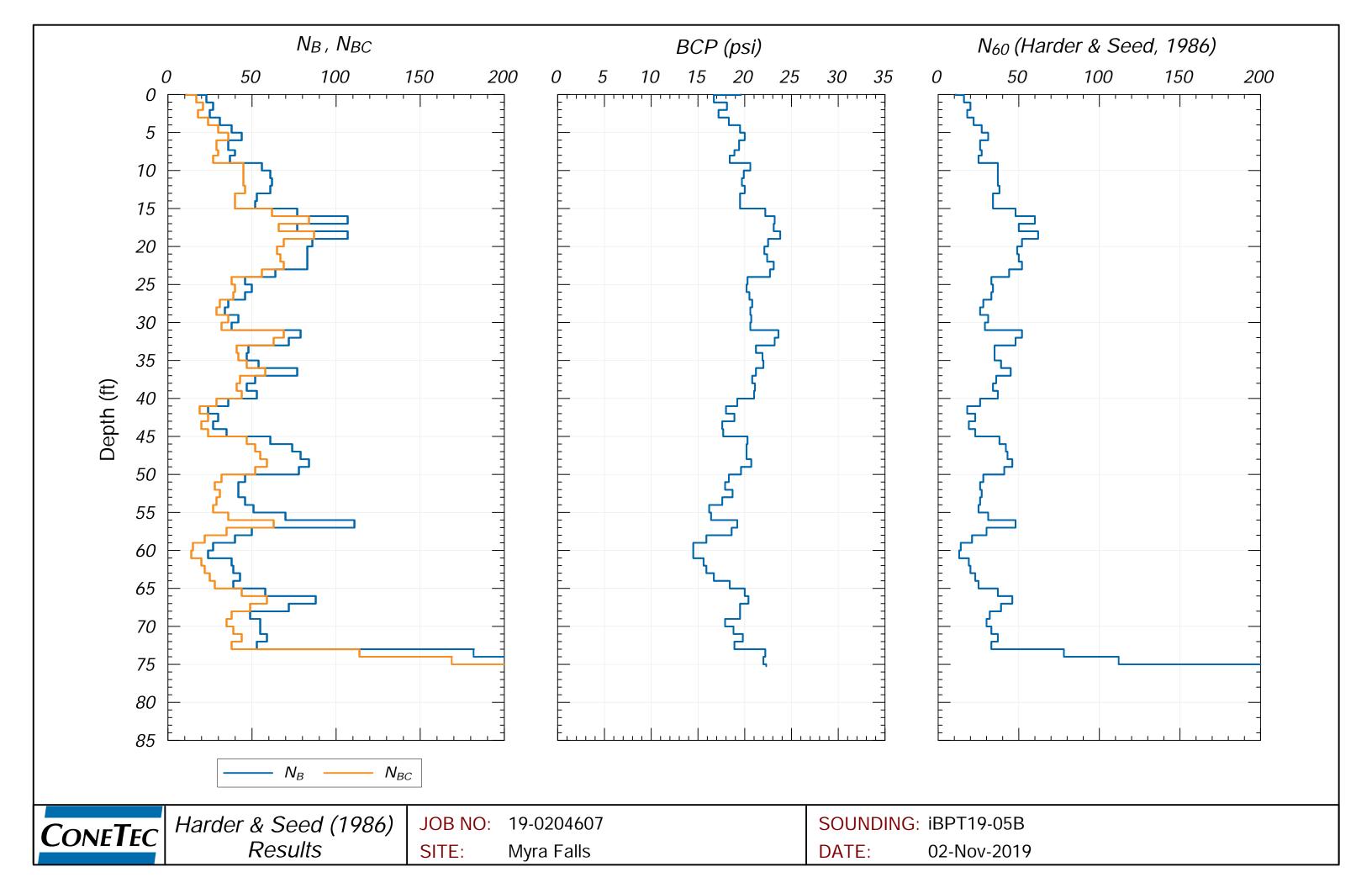
Instrumented Becker Penetration Test Plots with Harder & Seed (1986)
Results

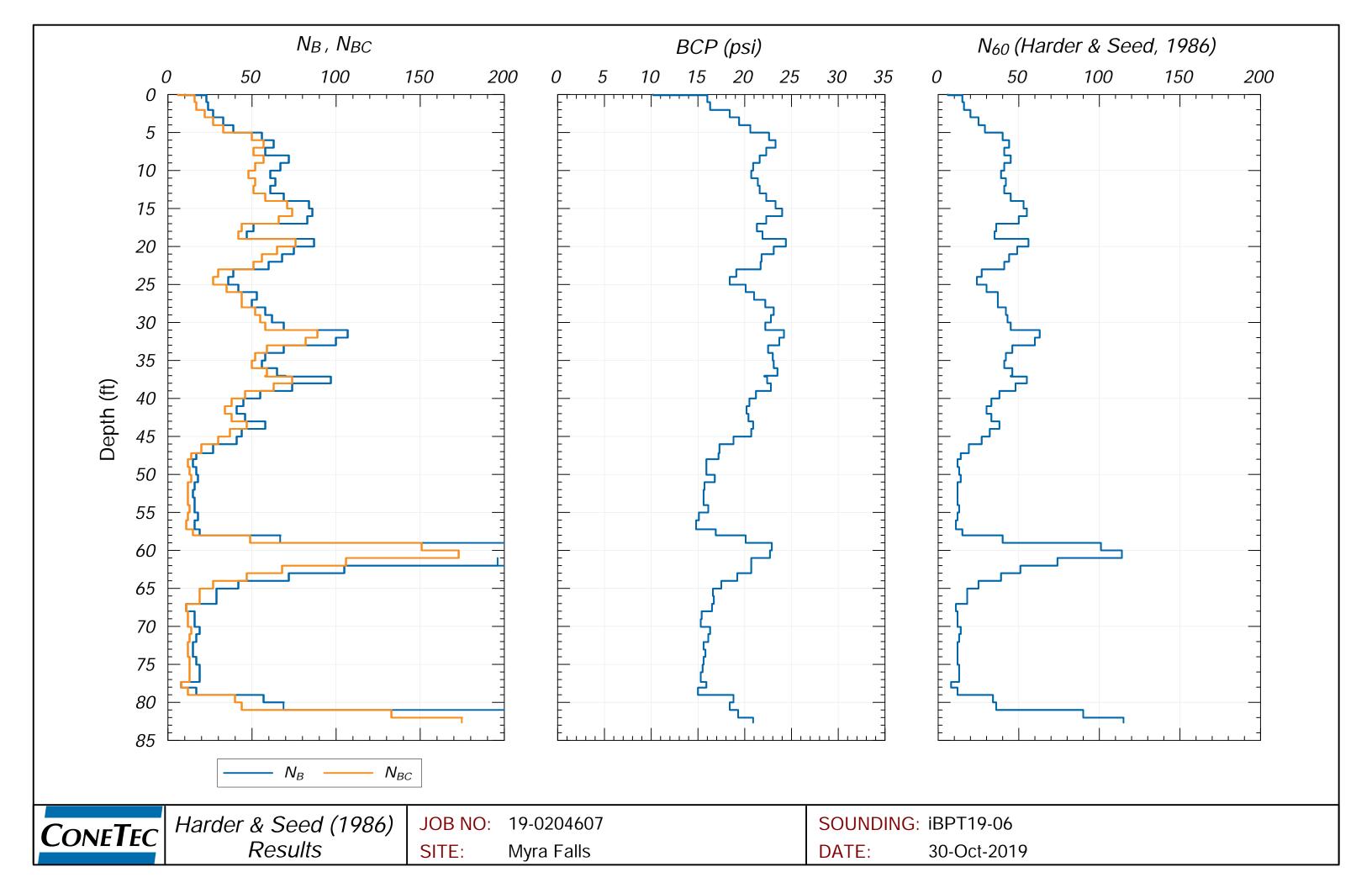












wood.

Appendix B – Geophysical Testing Report

PRESENTATION OF SITE INVESTIGATION RESULTS

Myra Falls Investigation

Prepared for:

Wood PLC

ConeTec Job No: 19-0204607

Project Start Date: 25-Nov-2019 Project End Date: 01-Dec-2019 Report Date: 18-Dec-2019



Prepared by:

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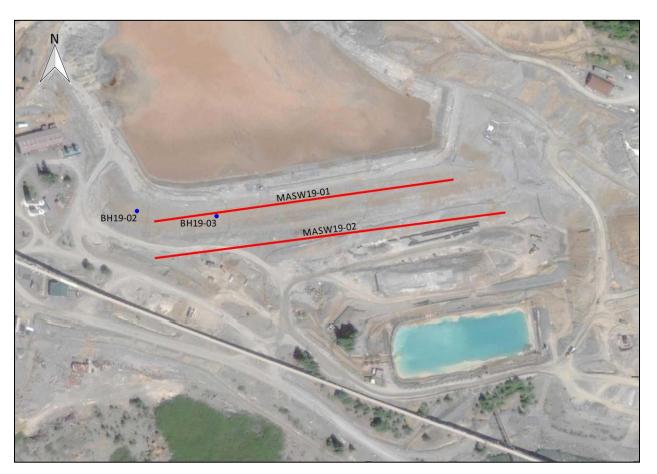
Introduction

The enclosed report presents the results of the geophysical site investigation program conducted by ConeTec Investigations Ltd., for Wood PLC, at the Myra Falls mine. The program consisted of two, two-dimensional Multichannel Analysis of Surface Waves (2D MASW) profiles and two Downhole Seismic Tests (DST).

Project Information

Project	
Client	Wood PLC
Project	Myra Falls Investigation
ConeTec project number	19-0204607

An aerial overview showing the MASW lines and DST locations is presented below.





Coordinates											
Test Type	Collection Method	EPSG Number	Comments								
MASW, DST	GNSS	6653	Coordinates were provided by Mifflin Surveys and are referenced to NAD83 datum, UTM Zone 10 North projection. Elevations were also provided by Mifflin Surveys. Depths are referenced to the ground surface at time of testing.								

MASW Acquisition Procedures

The MASW data was collected using a 72-channel static geophone array with geophones spaced at 2m. The active seismic source was a sledge hammer impacting vertically on an aluminum plate placed on the ground. At each shot location at least five strikes were recorded and added together (stacked) in the seismograph to produce a single seismic time domain trace record. The time domain records are 1 second in length with samples every 0.125 milliseconds (8 kHz). The source was then advanced 4m and the impacts were repeated, until the source reached the other side of the static array. After the shot location had advanced through the entire static array, the 24 channels furthest from the shot location were picked up and placed ahead of the current shot location. The data collection was repeated until the shot location had advanced, once again, through the static array. This procedure of advancing the shot then advancing part of the array is referred to as the roll-along method.

In addition to the active source seismic records, passive source seismic records were also collected for each array setup. The passive seismic records consisted of individual stack 30 second recordings sampled at 2 millisecond (500 Hz) intervals of ambient (background) seismic waves.

Seismograph record timing was initiated with a piezoelectric style trigger attached directly to the hammer that senses the vibration of impact with the plate. For each seismic record the field operator documented the seismic record number and location of the shot relative to the geophone station number. The equipment used for this project is outlined in the table below.

MASW Equipment Used for this Project									
Seismographs	Geophones	Geophones Coupling Mechanism		Seismic Sources					
3 x Geometrics	72 x Geospace	Aluminium pucks	piezoelectric	10 lb sledge hammer					
Geode 24	4.5 Hz vertical	and spikes	piezoeiectric						

MASW Data

Data quality was generally very good on both lines. To generate 2D shear wave velocity profiles with a 72-channel static geophone array each time domain record was re-sampled to a subset of 45 geophones oriented ahead of the shot location. This technique simulates a 45-channel geophone array with a 4m source offset and 4m array moves.

Overtone images of the 45-channel active source time domain records were generated and revealed coherent surface wave energy in the 10-60 Hz frequency band. Overtone images for the passive source time domain records were also produced and added further coherent surface wave energy in the 5-12



Hz band. The active and passive source Overtone images were then combined to produce an improved dataset. Examples of the 45 channel time domain traces and resulting combined (active and passive) Overtone images with picked dispersion curves are available in the appendices of this report.

Georeferencing of the shear wave velocity data was completed using the as-built survey data (start and end points) provided by Mifflin Surveys. Each geophone position was calculated by interpolating the X, Y, Z position between the surveyed end points with the help of the geophone spacing measured in the field. Each resulting Vs data point was then paired with the associated X, Y position. The Vs data was converted from Depth(m) to Elevation(m) by using the interpolated surface elevation for the corresponding X, Y position.

MASW Results

The 2D shear waves velocity profiles are shown in the appendices of this report. Each figure shows the 2D shear wave velocity profiles for that location along with an aerial image with the line placements and orientations. Included with the digital contents of this report are CSV files of the 2D shear wave velocity results containing columns of: Horizontal Distance(m), Easting(m), Northing(m), Depth(m), Vs(m/s). The results are georeferenced to NAD83 datum, UTM Zone 10 North projection (EPSG 6653). The datum of the vertical reference was not known at the time of writing.

DST Acquisition and Data Quality

The DST data was collected inside the 6" Sonic casing, instead of the more traditional method of using grouted in PVC. The testing interval was reduced from 1m to 0.5m for the bottom 11m of the borehole at the client's request. Both shear wave (Vs) and compression wave (Vp) data was collected for each borehole. The table below outlines the survey parameters.

Downhole Seismic Test (DST)					
Depth reference	Ground surface at time of testing				
Recorded depth interval	0.5 – 1m				
Sampling frequency and length	48 kHz for 300 milliseconds				
Seismic source	Beam (Vs) and Plate (Vp)				

The data quality ranged from excellent to very poor. For BH19-02 data below 35m (below ground surface) was excellent, however above 35m the data quality degraded significantly. This can be seen in the time domain traces as significant ringing in the signal after the first wave arrival. This ringing is not seen in the bottom 4m of data in BH19-02. The ringing is likely a result of poor coupling between the Sonic casing and surrounding soils. As a result, picking a seismic arrival time at each depth was not possible. Data quality for BH19-03 was similar to the top portion of BH19-02. Data quality was particularly bad below 28m, resulting in fewer interpretable time arrivals in this section.

The DST results are presented in the appendices of this report. The results are also included in the digital release in XLSX format.



Limitations

This report has been prepared for the exclusive use of Wood PLC (Client) for the project titled "Myra Falls Investigation". The report's contents may not be relied upon by any other party without the express written permission of ConeTec Investigations Ltd. (ConeTec). ConeTec has provided site investigation services, prepared the factual data reporting and provided geotechnical parameter calculations consistent with current best practices. No other warranty, expressed or implied, is made.

The information presented in the report document and the accompanying data set pertain to the specific project, site conditions and objectives described to ConeTec by the Client. In order to properly understand the factual data, assumptions and calculations, reference must be made to the documents provided and their accompanying data sets, in their entirety.



Multichannel analysis of surface waves (MASW) is a non-intrusive in-situ test that uses the principles of elasticity and surface wave dispersion to determine the variation of shear wave velocity with depth at a site. The observation that surface waves (Rayleigh waves) of different wavelengths propagate at different phase velocities in non-ideal media, is called dispersion. This is a direct result of the fact that surface waves of different wavelengths propagate along the surface to varying depths, and hence, if material stiffness changes with depth (as is the case with most non-ideal materials), then an appropriately selected wavelength band will reflect such changes in the velocity of propagation.

The field methods for surface wave testing are very similar to other surface seismic data collection methods. Surface geophones are placed in a linear array along a survey line at a known separation (typically one metre). A series of recordings (shots) are collected with a known in-line source offset from the array. Each shot gather is represented in the time-offset domain and shows the amplitude of wave propagation through the array (refer to Figure MASW-1). For detailed frequency analysis, multiple records with different shot offset distances are collected to help better define the broad spectrum frequency-phase velocity response of the medium. Two-dimensional cross sections can be collected by moving the geophone array a small distance (typically two meters) along the line and repeating the shots at set offsets.

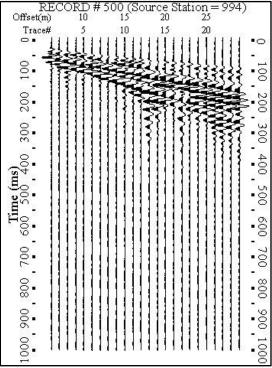


Figure MASW-1. Typical MASW time domain record (shot gather)

Given that surface wave velocity is closely related to the shear wave velocity and the wavelength related to depth, the surface wave results can be used to develop a profile of shear wave velocity versus depth through a process referred to as inversion. The program used to perform the inversion is SurfSeis 6.6, developed by the Kansas Geological Survey. In SurfSeis, the raw time domain traces are transformed to the frequency domain to create what is referred to as an overtone image as shown in Figure MASW-2. The overtone image displays the amplitude of the primary surface wave mode and any potential higher modes. A dispersion curve is fitted to the overtone image, and the inversion process is then used to



determine the most appropriate shear wave velocity profile. The parameters used for the inversion of the dispersion data are provided in the data release folder in an Excel table.

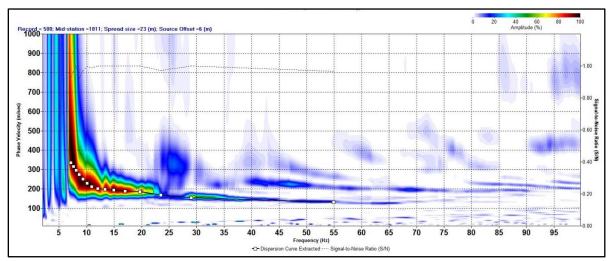


Figure MASW-2. Overtone image and a picked dispersion curve

For each test location, a 1D shear wave velocity profile comprising of a number of velocity layers of variable thickness (refer to Figure MASW-3) is provided. For 2D testing a series of 1D tests are combined to produce a shear wave velocity cross section.

The depth of investigation is related to the ground conditions and the amount of energy delivered by the surface wave source. The surface wave method uses Rayleigh waves that travel horizontally along the ground surface to a depth of about one wavelength. The actual depth of sampling of the ground is considered to be one-half to one-third of the Rayleigh (surface) wave wavelength. The wavelengths measured by the equipment will be a function of the frequency of the source and the velocity of the surface waves through the ground. As the depth of investigation increases, there will be less certainty in terms of layer boundaries and velocity values.

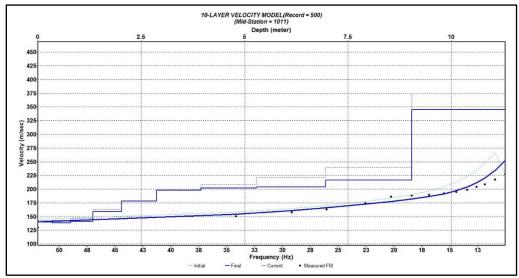


Figure MASW-3. 1D inversion result with fitted dispersion curve



The equipment, field procedures, and analysis software used by ConeTec all conform to the currently accepted best practices for MASW testing. The results of geophysical testing are always interpretative to a certain extent and should be confirmed by drilling or other intrusive testing.

References

Miller, R.D., Xia, J., Park, C.B., and Ivanov, J.M., 1999, Multichannel analysis of surface waves to map bedrock, Kansas Geological Survey, The Leading Edge, December, p. 1392-1396.

Park, C.B., Miller, R.D., and Xia, J., 1998b, Ground roll as a tool to image near-surface anomaly: 68th Ann. Internat. Mtg. Soc. Expl.Geophys., Expanded Abstracts, p. 874-877.

Park, C.B., Miller, R.D., and Xia, J., 1999, Multichannel analysis of surface waves: Geophysics, v. 64, n. 3, pp. 800-808.

Park, C.B., Miller, R.D., Xia, J., and Ivanov, J., 2007, Multichannel analysis of surface waves (MASW)-active and passive methods: The Leading Edge, January.

SurfSeis website: http://www.kgs.ku.edu/software/surfseis/index.html

Xia, J., R.D. Miller, and C.B. Park, 2000a, Advantages of calculating shear-wave velocity from surface waves with higher modes: [Exp. Abs.]: Soc. Expl. Geophys., p. 1295-1298.

Xia, J., Miller, R.D., Park, C.B., and Ivanov, J., 2000b, Construction of 2-D vertical shear-wave velocity field by the Multichannel Analysis of Surface Wave technique, Proceedings of the Symposium on the Application of Geophysics to Engineering and Environmental Problems (SAGEEP 2000), Washington D.C, February 20-24, p. 1197-1206.



Downhole seismic testing (DST) is conducted using a system comprising of a surface source, a downhole tool equipped with a triaxial geophone package, and a data acquisition system.

The downhole tool has a triaxial geophone package mounted on an internal block such that the orientation of the geophones can be maintained within the borehole through the use of the built in fluxgate compass and servo motor system. A motor driven bow spring clamp is used to couple the downhole tool with the borehole wall. The downhole seismic test equipment is in general accordance with the current ASTM D7400 standard.

Shear waves (Vs) are typically generated by using an impact hammer horizontally striking a beam that is held in place by a normal load while compression waves (Vp) are typically generated by using an impact hammer vertically striking a metal plate. The hammer and beam (or plate) act as a contract trigger that initiates the recording of the seismic wave traces. The beam is generally struck on each end to generate horizontally polarized shear waves. The traces are recorded using an uphole data acquisition system. An illustration of the downhole seismic testing configuration is presented in Figure DHS-1.

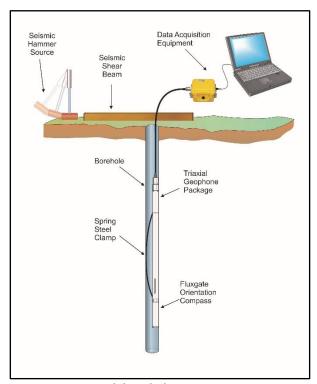


Figure DHS-1. Typical downhole seismic testing equipment

Prior to conducting downhole seismic testing, the horizontal offset between the borehole and seismic source is measured and recorded.

Testing is performed by lowering the tool into a cased borehole to the initial start depth where it is coupled to the side of the casing using the spring clamp. The two horizontal geophones are aligned parallel and perpendicular to the shear wave source. Doing so maintains the same orientation of the geophones throughout the test, which eliminates any apparent phase changes due to rotation of the tool.



Multiple wave traces are recorded and reviewed on the data acquisition computer. Once sufficient data has been recorded, the tool is lowered to the next depth by a set increment (typically one meter) and the procedures are repeated. Test procedures are in general accordance with the current ASTM D7400 standard.

Determination of interval travel times are performed by visually picking a common feature (e.g. the first characteristic peak, trough, or crossover) on all of the recorded wave sets. The velocity can then be determined by taking the difference in ray path divided by the interval travel time. The ray path is defined as the straight line distance from the seismic source to the geophone; accounting for the source offset and source depth. Alternatively, the velocity can be determined by calculating the slope of the accumulated travel time – ray path data. This is normally done on a 3 or 5 point basis using linear least squares regression (Arsenault et al. (2012)). Doing so also allows for the calculation of the standard error which provides a qualitative indication of the velocity uncertainty determined from linear least squares regression.

A summary of the testing performed and the seismic wave velocity data, presented in tabular and graphical format is provided in the relevant appendix. Images of the time domain traces used for the shear wave and compression wave picks are presented for reference in an additional appendix. The traces provide a visual representation of the seismic data and an indication of the data quality.

References

ASTM D7400/D7400M-19, 2019, "Standard Test Methods for Downhole Seismic Testing", ASTM International, West Conshohocken, PA. DOI: 10.1520/D7400_D7400M-19.

Arsenault, J.-L., Hunter, J.A. and Crow, H.L., 2012. Shear Wave Velocity Logs From Vertical Seismic Profiles; *in* Shear Wave Velocity Measurement Guidelines for Canadian Seismic Site Characterization in Soil and Rock, (ed.) J.A. Hunter and H.L. Crow; Geological Survey of Canada, Open File 7078, p.123-138.



The appendices listed below are included in the report:

- 2D MASW Test Summary and Profiles
- MASW Time Domain Traces and Overtone Images
- DST Summary and Results
- DST Time Domain Traces



2D MASW Test Summary and Profiles





Job No: 19-0204607 Client: Wood PLC

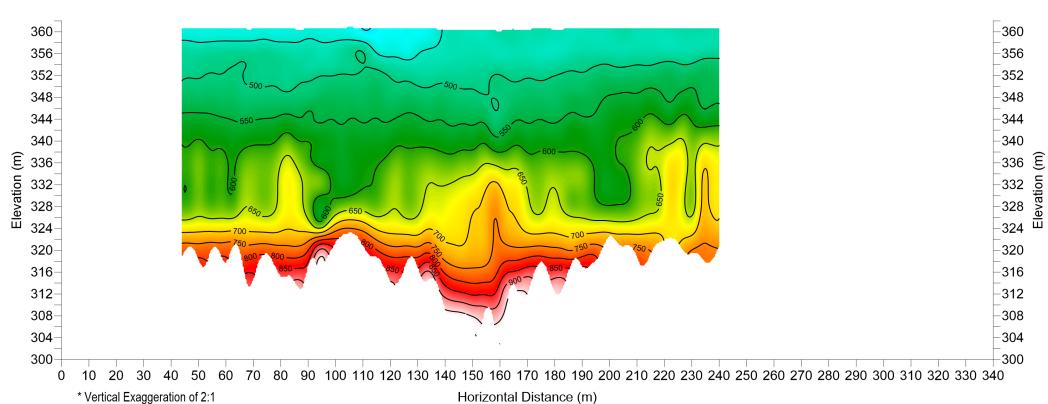
Project: Myra Falls Investigation

Start Date: 25-Nov-2019 End Date: 01-Dec-2019

2D MASW TEST SUMMARY											
Section ID	Location	Date(s)	Geophone Spacing (m)	Line Length (m)	Start of Section Northing ¹ (m)	Start of Section Easting (m)	End of Section Northing (m)	End of Section Easting (m)	Refer to Notation Number		
MASW19-01	Drill Bench	25-Nov-2019, 26-Nov-2019	2	286	5494646	311747	5494685	312030	2		
MASW19-02	Dam Face	27-Nov-2019, 28-Nov-2019	2	334	5494611	311748	5494650	312046	2		

^{1.} Coordinates were provided by Mifflin Surveys and are referenced to NAD83 datum, UTM Zone 10 North projection.

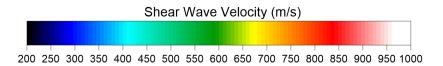
^{2.} Elevations were provided by Mifflin Surveys.



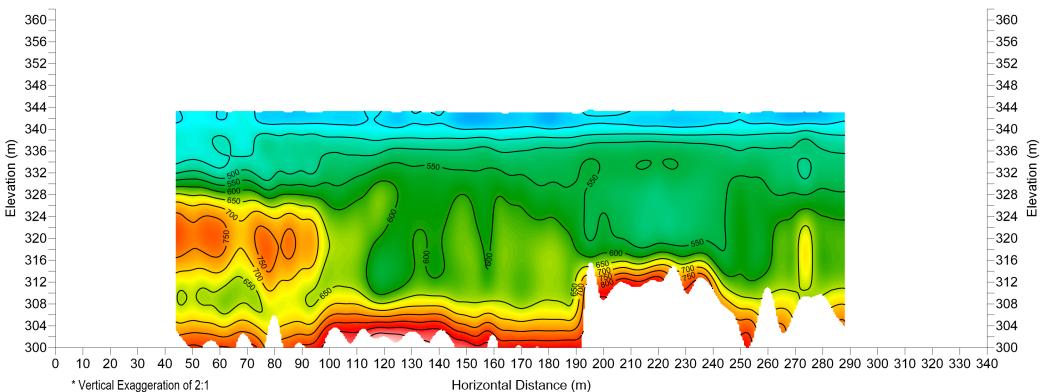
Myra Falls Investigation

FIGURE 1
MASW19-01: 44 to 240 m

INFORMATION TABLE						
HORIZONTAL DATUM	NAD 83					
PROJECTION	UTM Zone 10 North					
VERTICAL REFERENCE	Client Provided Elevation					
DATE(S) ISSUED / REVISED	16-DEC-2019					
SURVEY DATE(S)	25-NOV-2019 to 26-NOV-2019					
CONETEC JOB NUMBER	19-0204607					

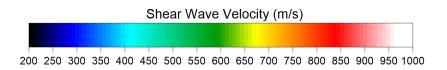








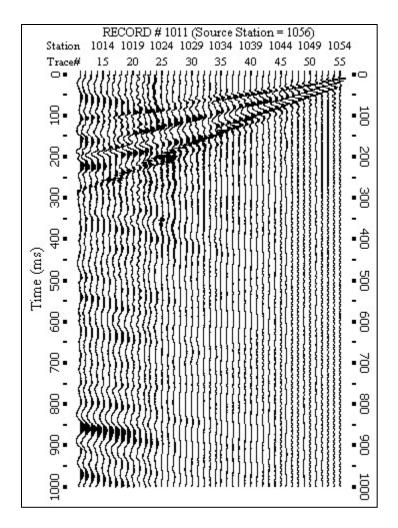
INFORMATION TABLE							
HORIZONTAL DATUM	NAD 83						
PROJECTION	UTM Zone 10 North						
VERTICAL REFERENCE	Client Provided Elevation						
DATE(S) ISSUED / REVISED	16-DEC-2019						
SURVEY DATE(S)	27-NOV-2019 to 28-NOV-2019						
CONETEC JOB NUMBER	19-0204607						

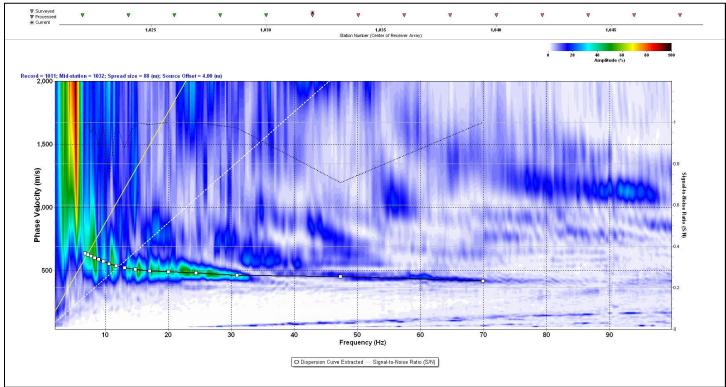




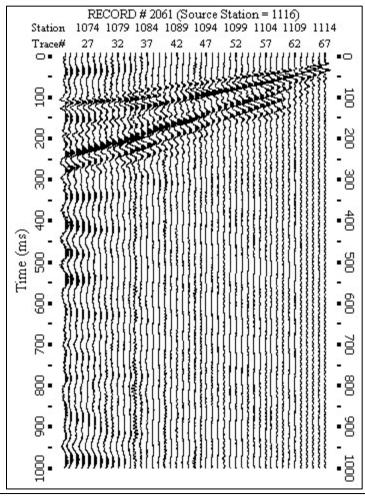
MASW Time Domain Traces and Overtone Images

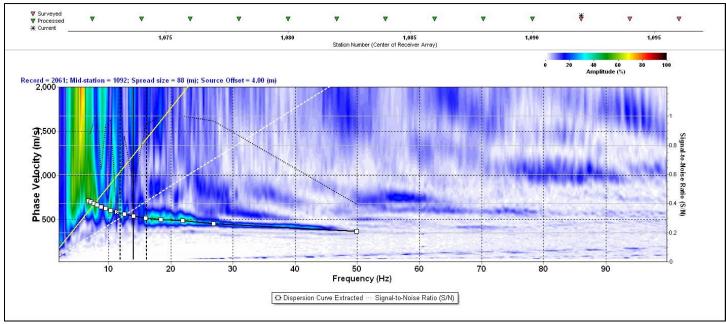






MASW19-01: example time domain trace (top) and overtone image with picked dispersion curve (bottom).





MASW19-02: example time domain trace (top) and overtone image with picked dispersion curve (bottom).

DST Summary and Results





Project: Myra Falls Investigation

Start Date: 25-Nov-2019 End Date: 01-Dec-2019

DOWNHOLE SEISMIC SUMMARY										
Sounding ID	Date	Final Depth (m)	Northing ¹ (m)	Easting ¹ (m)	Elevation ² (m)	Refer to Notation Number				
DST19-01	BH19-02	29-Nov-2019	38.25	5494655.6	311729.7	362.4				
DST19-02	BH19-03	01-Dec-2019	38.30	5494650.7	311805.5	362.4				

^{1.} Coordinates were provided by Mifflin Surveys and are referenced to NAD83 datum, UTM Zone 10 North projection.

^{2.} Elevations were provided by Mifflin Surveys and are referenced to the existing ground surface at the time of testing.



Project: Myra Falls Investigation

Sounding ID: BH19-02 Date: 29-Nov-2019

Receivers: Geostuff BHG-3 - Triaxial 15 Hz geophones

Seismograph: Geometrics Geode

Seismic Source: Beam
Source Offset (m): 0.70
Source Depth (m): 0.00
Geophone Offset (m): 0.00

DO	OWNHOLE SEISMI	C SHEAR WAVI	E VELOCITY TES	T RESULTS - V	/s
Geophone Depth (m)	Ray Path (m)	Ray Path Difference (m)	Travel Time Interval (ms)	Interval Velocity (m/s)	Average Interval Depth (m)
2.75	2.84				
3.68	3.75	0.91	1.78	511	3.2
4.75	4.80	1.06	2.28	464	4.2
6.75	6.79	1.98	3.99	498	5.8
7.75	7.78	1.00	2.05	486	7.3
8.75	8.78	1.00	2.20	454	8.3
10.75	10.77	1.99	3.64	548	9.8
11.75	11.77	1.00	1.71	584	11.3
13.75	13.77	2.00	3.38	592	12.8
14.75	14.77	1.00	1.91	522	14.3
15.75	15.77	1.00	1.84	543	15.3
16.25	16.27	0.50	0.86	578	16.0
16.75	16.76	0.50	0.65	774	16.5
17.25	17.26	0.50	0.62	799	17.0
18.25	18.26	1.00	1.35	740	17.8
18.75	18.76	0.50	0.67	743	18.5
19.25	19.26	0.50	0.64	784	19.0
19.75	19.76	0.50	0.60	831	19.5
20.25	20.26	0.50	0.63	799	20.0
20.75	20.76	0.50	0.71	706	20.5
21.25	21.26	0.50	0.73	686	21.0
21.75	21.76	0.50	0.73	686	21.5
22.25	22.26	0.50	0.73	680	22.0
23.25	23.26	1.00	1.35	739	22.8
24.75	24.76	1.50	2.41	621	24.0



Project: Myra Falls Investigation

Sounding ID: BH19-02 Date: 29-Nov-2019

Receivers: Geostuff BHG-3 - Triaxial 15 Hz geophones

Seismograph: Geometrics Geode

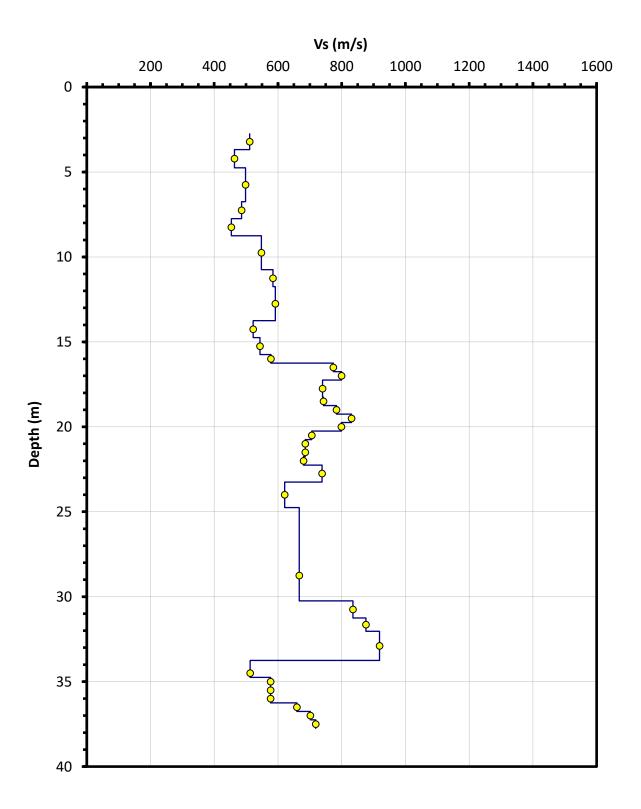
Seismic Source: Beam
Source Offset (m): 0.70
Source Depth (m): 0.00
Geophone Offset (m): 0.00

DO	DOWNHOLE SEISMIC SHEAR WAVE VELOCITY TEST RESULTS - Vs									
Geophone Depth (m)	Ray Path (m)	Ray Path Difference (m)	Travel Time Interval (ms)	Interval Velocity (m/s)	Average Interval Depth (m)					
30.25	30.26	3.00	4.50	667	28.8					
31.25	31.26	1.00	1.20	836	30.8					
32.04	32.05	0.79	0.90	876	31.6					
33.75	33.76	1.71	1.86	919	32.9					
34.75	34.76	0.50	0.98	513	34.5					
35.25	35.26	0.50	0.87	577	35.0					
35.75	35.76	0.50	0.87	577	35.5					
36.25	36.26	0.50	0.87	577	36.0					
36.75	36.76	0.50	0.76	659	36.5					
37.25	37.26	0.50	0.71	702	37.0					
37.75	37.76	0.50	0.70	718	37.5					



Project: Myra Falls Investigation

Sounding ID: BH19-02 Date: 29-Nov-2019





Project: Myra Falls Investigation

Sounding ID: BH19-02 Date: 29-Nov-2019

Receivers: Geostuff BHG-3 - Triaxial 15 Hz geophones

Seismograph: Geometrics Geode

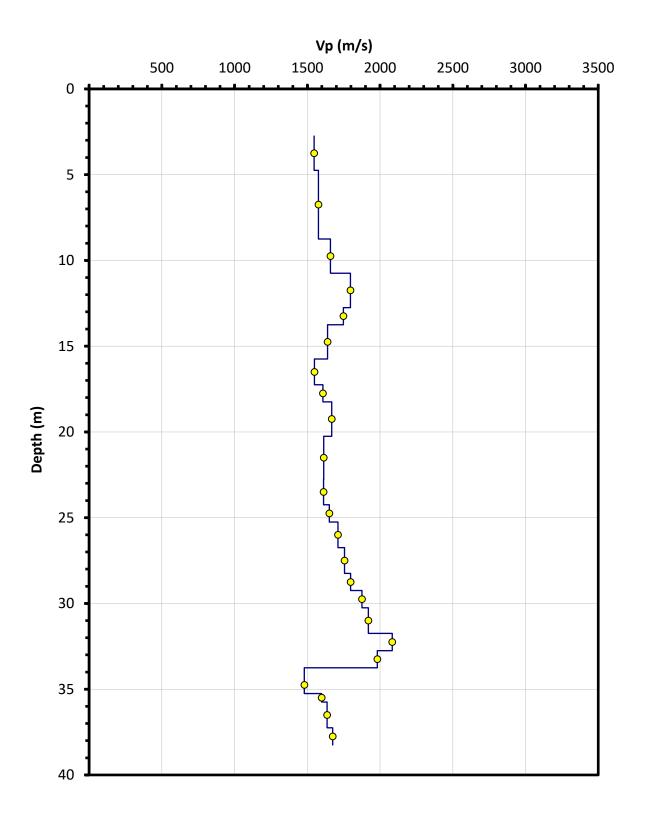
Seismic Source: Plate
Source Offset (m): 1.85
Source Depth (m): 0.00
Geophone Offset (m): 0.00

DOWNF	DOWNHOLE SEISMIC COMPRESSION WAVE VELOCITY TEST RESULTS - Vp									
Geophone Depth (m)	Ray Path (m)	Ray Path Difference (m)	Travel Time Interval (ms)	Interval Velocity (m/s)	Average Interval Depth (m)					
2.75	3.31									
4.75	5.10	1.78	1.15	1546	3.8					
8.75	8.94	3.85	2.44	1577	6.8					
10.75	10.91	1.96	1.18	1659	9.8					
12.75	12.88	1.98	1.10	1796	11.8					
13.75	13.87	0.99	0.57	1748	13.3					
15.75	15.86	1.98	1.21	1639	14.8					
17.25	17.35	1.49	0.96	1549	16.5					
18.25	18.34	0.99	0.62	1607	17.8					
20.25	20.33	1.99	1.19	1668	19.3					
22.75	22.83	2.49	1.54	1612	21.5					
24.25	24.32	1.50	0.93	1612	23.5					
25.25	25.32	1.00	0.60	1651	24.8					
26.75	26.81	1.50	0.87	1710	26.0					
28.25	28.31	1.50	0.85	1756	27.5					
29.25	29.31	1.00	0.56	1797	28.8					
30.25	30.31	1.00	0.53	1876	29.8					
31.75	31.80	1.50	0.78	1919	31.0					
32.75	32.80	1.00	0.48	2084	32.3					
33.75	33.80	1.00	0.50	1981	33.3					
35.25	35.30	1.00	0.68	1479	34.8					
35.75	35.80	0.50	0.31	1598	35.5					
37.25	37.30	1.50	0.92	1636	36.5					
38.25	38.29	1.00	0.60	1674	37.8					



Project: Myra Falls Investigation

Sounding ID: BH19-02 Date: 29-Nov-2019





Project: Myra Falls Investigation

Sounding ID: BH19-03 Date: 01-Dec-2019

Receivers: Geostuff BHG-3 - Triaxial 15 Hz geophones

Seismograph: Geometrics Geode

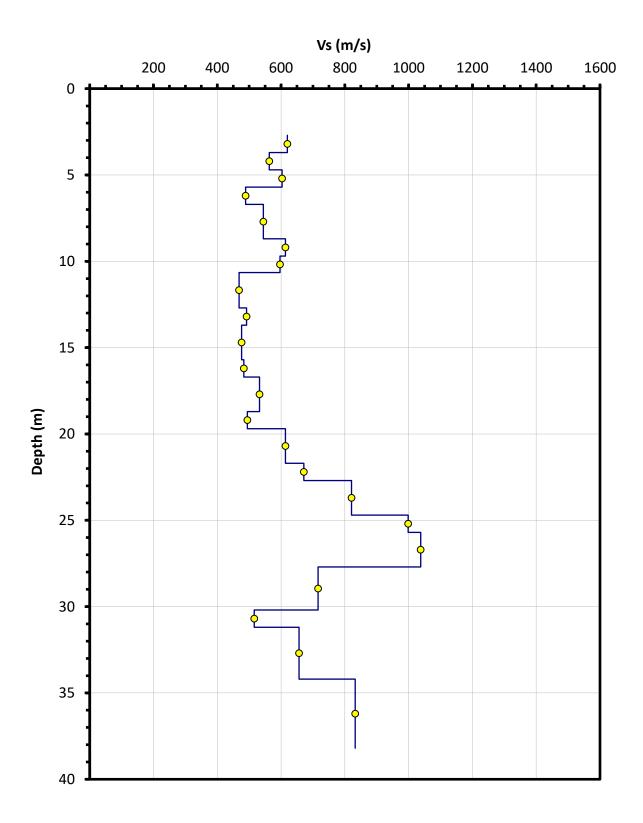
Seismic Source: Beam
Source Offset (m): 0.80
Source Depth (m): 0.00
Geophone Offset (m): 0.00

DO	DOWNHOLE SEISMIC SHEAR WAVE VELOCITY TEST RESULTS - Vs								
Geophone Depth (m)	Ray Path (m)	Ray Path Difference (m)	Travel Time Interval (ms)	Interval Velocity (m/s)	Average Interval Depth (m)				
2.70	2.82								
3.70	3.79	0.97	1.56	620	3.2				
4.70	4.77	0.98	1.74	563	4.2				
5.70	5.76	0.99	1.64	603	5.2				
6.70	6.75	0.99	2.03	489	6.2				
8.70	8.74	1.99	3.65	544	7.7				
9.70	9.73	1.00	1.62	613	9.2				
10.65	10.68	0.95	1.59	597	10.2				
12.70	12.73	2.05	4.37	468	11.7				
13.70	13.72	1.00	2.03	492	13.2				
15.70	15.72	2.00	4.20	476	14.7				
16.70	16.72	1.00	2.07	483	16.2				
18.70	18.72	2.00	3.75	532	17.7				
19.70	19.72	1.00	2.02	494	19.2				
21.70	21.71	2.00	3.26	614	20.7				
22.70	22.71	1.00	1.49	671	22.2				
24.70	24.71	2.00	2.43	821	23.7				
25.70	25.71	1.00	1.00	998	25.2				
27.70	27.71	2.00	1.93	1038	26.7				
30.20	30.21	2.50	3.49	716	29.0				
31.20	31.21	1.00	1.94	516	30.7				
34.20	34.21	3.00	4.57	657	32.7				
38.20	38.21	4.00	4.81	832	36.2				



Project: Myra Falls Investigation

Sounding ID: BH19-03
Date: 1-Dec-2019





Project: Myra Falls Investigation

Sounding ID: BH19-03 Date: 01-Dec-2019

Receivers: Geostuff BHG-3 - Triaxial 15 Hz geophones

Seismograph: Geometrics Geode

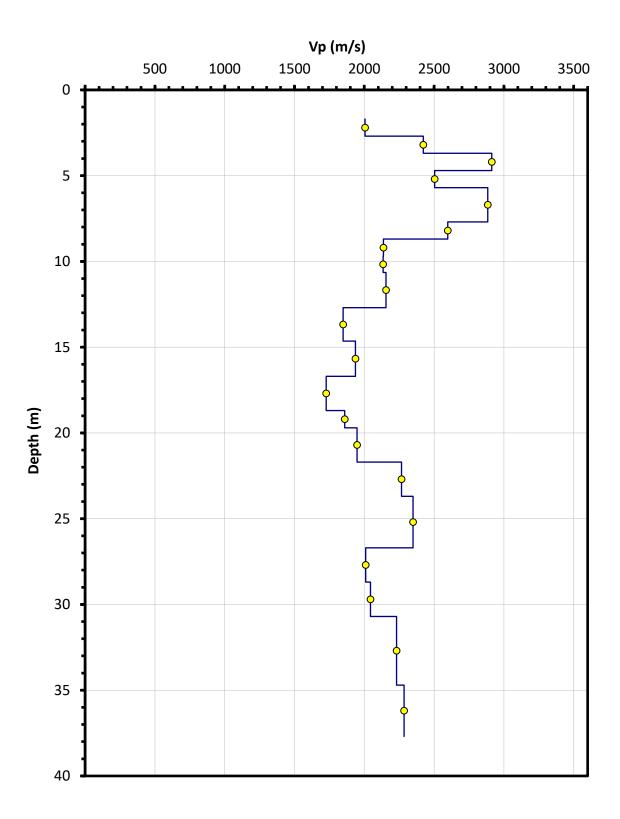
Seismic Source: Plate
Source Offset (m): 1.90
Source Depth (m): 0.00
Geophone Offset (m): 0.00

Geophone	Ray	Ray Path	Travel Time	Interval	Average
Depth	, Path	Difference	Interval	Velocity	Interval Dept
(m)	(m)	(m)	(ms)	(m/s)	(m)
1.70	2.55				
2.70	3.30	0.75	0.37	2006	2.2
3.70	4.16	0.86	0.35	2422	3.2
4.70	5.07	0.91	0.31	2913	4.2
5.70	6.01	0.94	0.37	2504	5.2
7.70	7.93	1.92	0.67	2884	6.7
8.70	8.91	0.97	0.37	2598	8.2
9.70	9.88	0.98	0.46	2137	9.2
10.65	10.82	0.93	0.44	2135	10.2
12.70	12.84	2.02	0.94	2156	11.7
14.65	14.77	1.93	1.05	1848	13.7
16.70	16.81	2.04	1.05	1936	15.7
18.70	18.80	1.99	1.15	1727	17.7
19.70	19.79	1.00	0.54	1860	19.2
21.70	21.78	1.99	1.02	1948	20.7
23.70	23.78	1.99	0.88	2267	22.7
26.70	26.77	2.99	1.27	2349	25.2
28.70	28.76	2.00	0.99	2009	27.7
30.70	30.76	2.00	0.98	2044	29.7
34.70	34.75	3.99	1.79	2231	32.7
37.70	37.75	3.00	1.31	2284	36.2



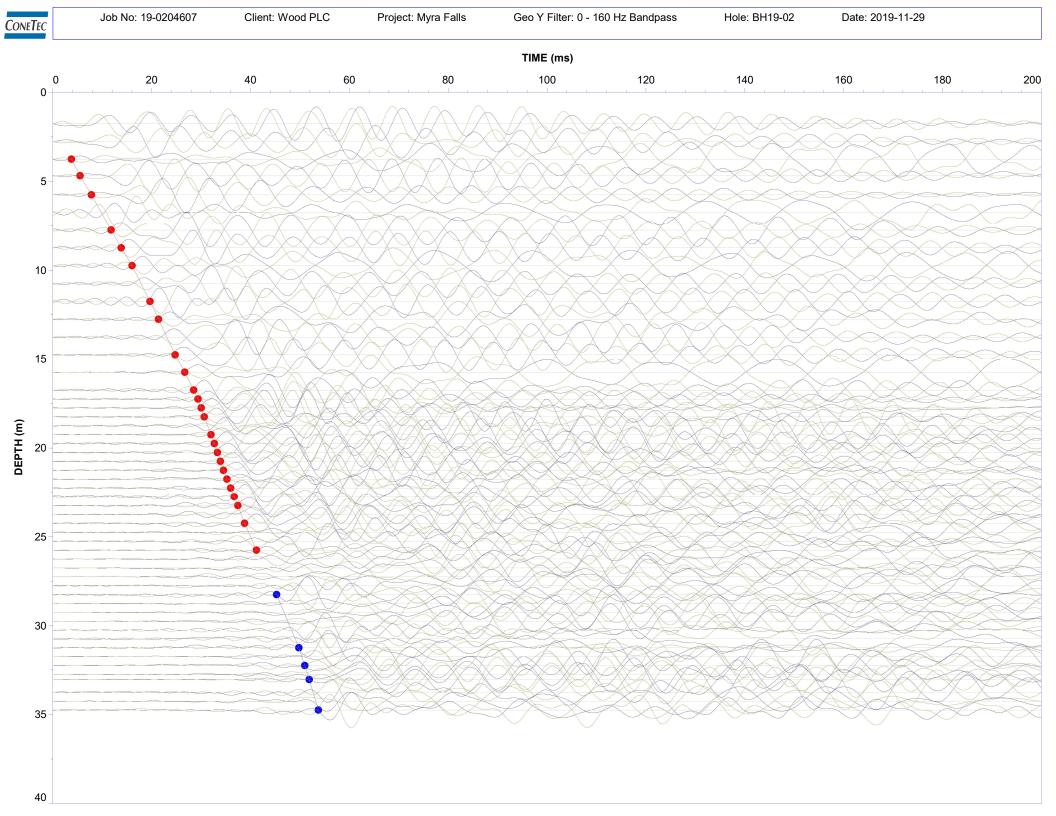
Project: Myra Falls Investigation

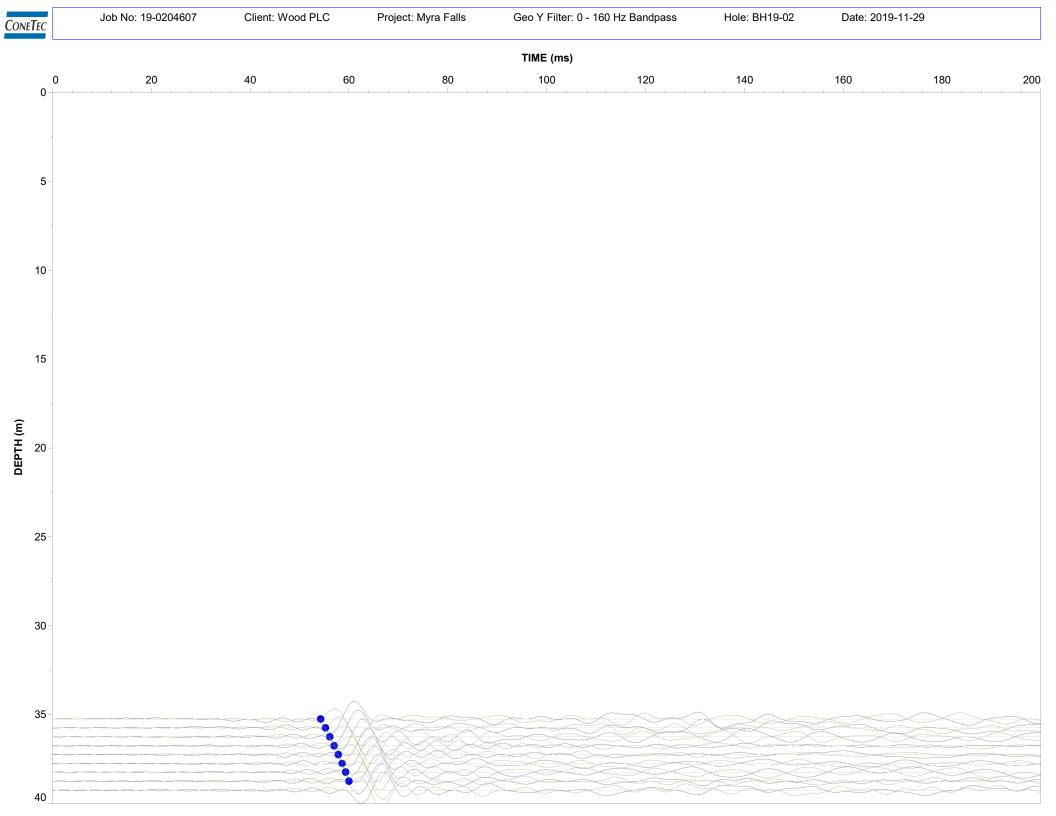
Sounding ID: BH19-03 Date: 1-Dec-2019

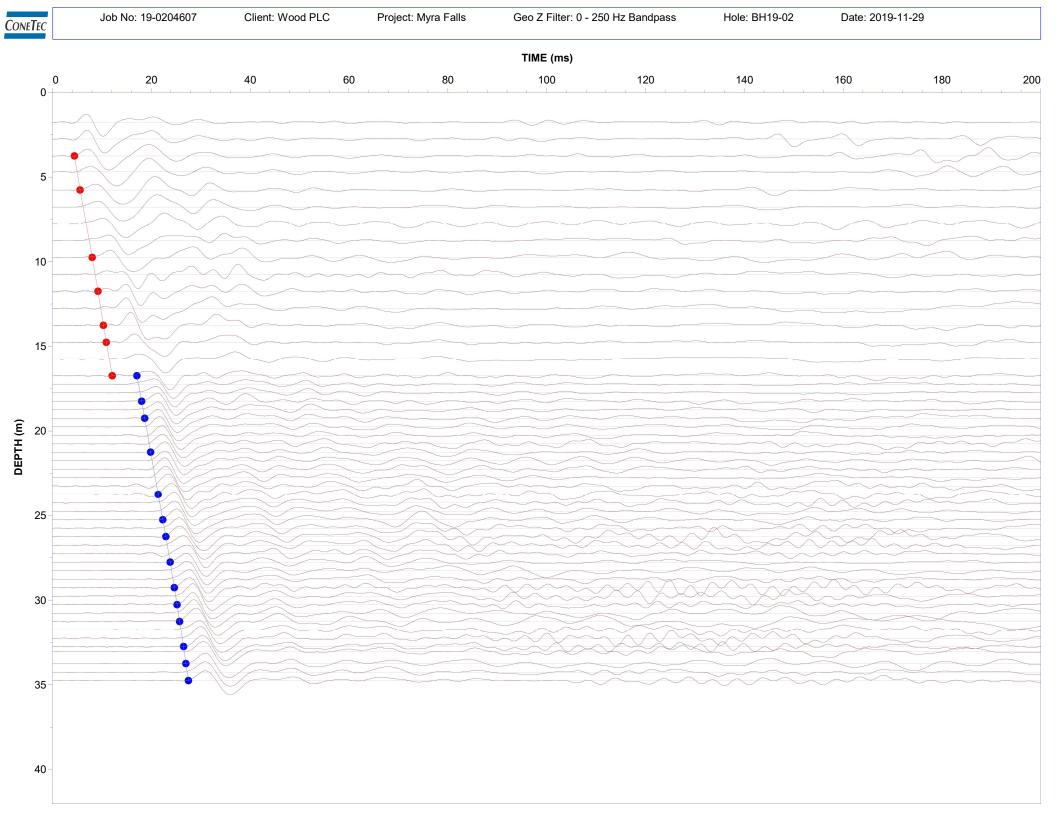


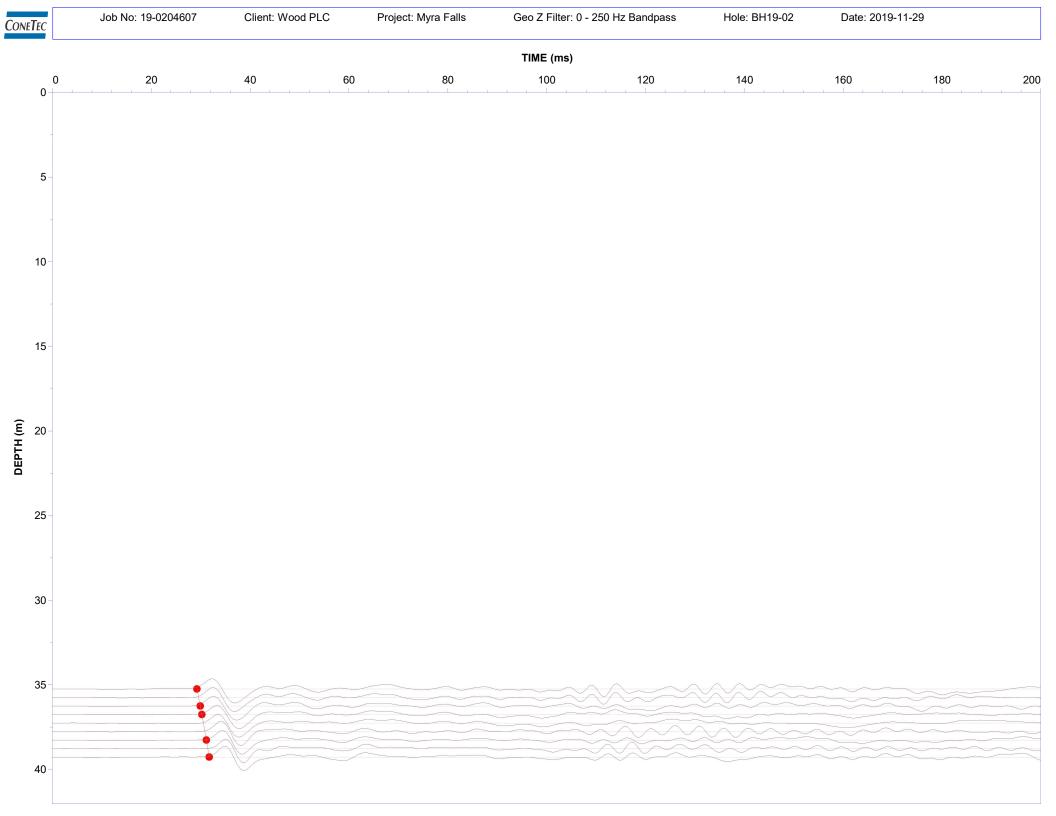
DST Time Domain Traces

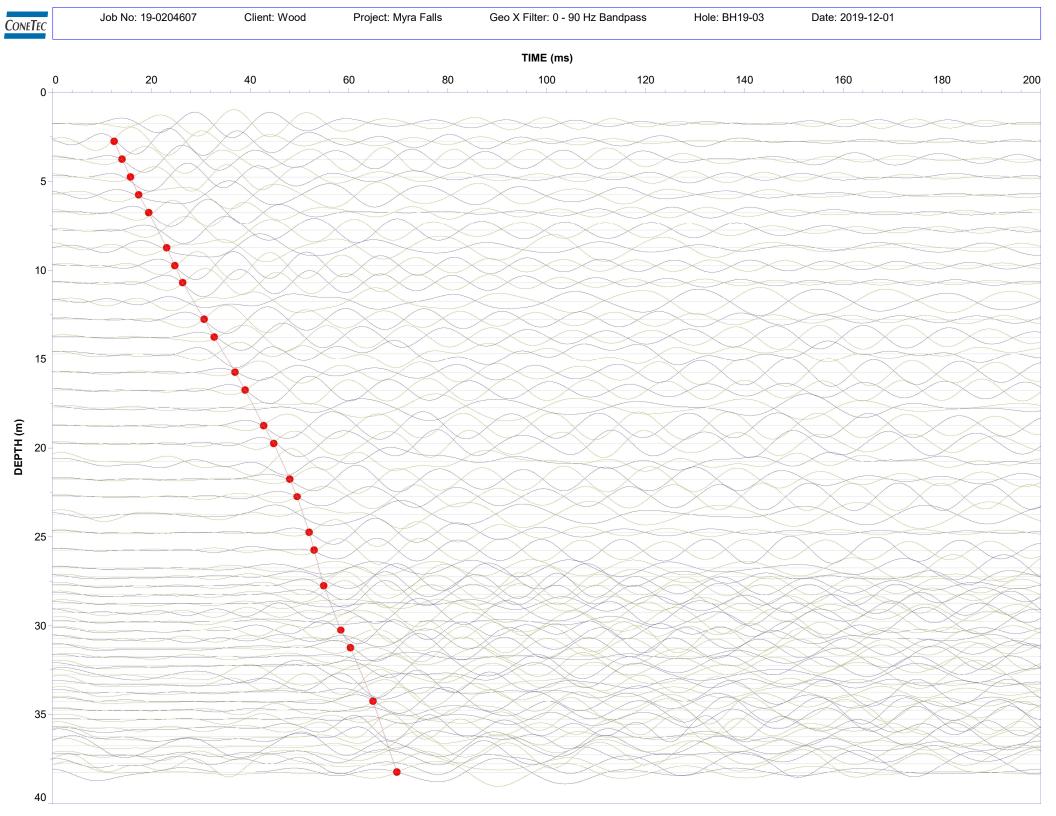


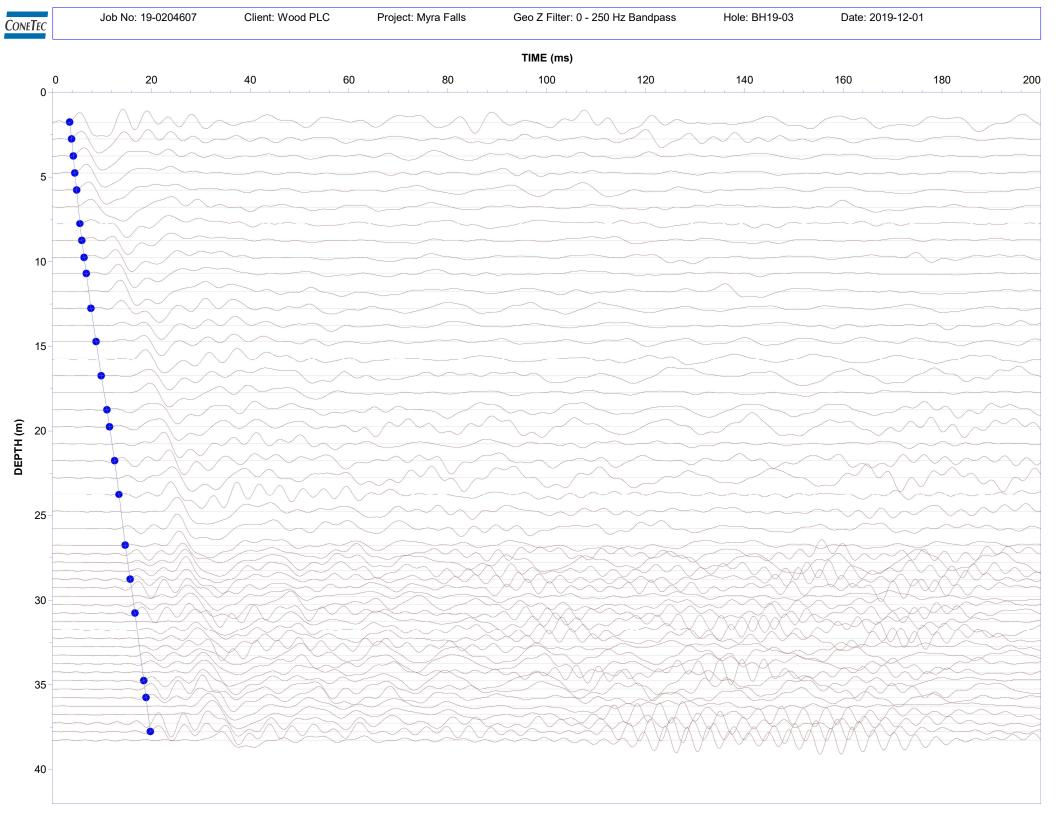












wood.

Appendix C – Borehole Logs

EXPLANATION OF TERMS AND SYMBOLS

The terms and symbols used on the borehole logs to summarize the results of field investigation and subsequent laboratory testing are described in these pages.

It should be noted that materials, boundaries and conditions have been established only at the borehole locations at the time of investigation and are not necessarily representative of subsurface conditions elsewhere across the site.

TEST DATA

Data obtained during the field investigation and from laboratory testing are shown at the appropriate depth interval.

Abbreviations, graphic symbols, and relevant test method designations are as follows:

*C	Consolidation Test	TV	Torvane shear strength
D_R	Relative Density	VS	Vane shear strength
*k	Permeability coefficient	W	Natural Moisture Content (ASTM D2216)
*MA	Mechanical grain size analysis	wl	Liquid Limit (ASTM D 423)
	and hydrometer test	W_p	Plastic Limit (ASTM D 424)
N	Standard Penetration Test	Ef	Unit strain at failure
	(CSA A119.1-60)	γ	Unit weight of soil or rock
N_{d}	Dynamic cone penetration test	γd	Dry unit weight of soil or rock
NP	Non plastic soil	ρ	Density of soil or rock
Pp	Pocket penetrometer strength	ρ d	Dry Density of soil or rock
*q	Triaxial compression test	C_{u}	Undrained shear strength
q_{u}	Unconfined compressive strength	\rightarrow	Seepage
*SB	Shearbox test	<u>▼</u>	Observed water level
SO_4	Concentration of water-soluble sulphate	$\overline{ abla}$	Water level at completion of drilling
	* The area it a ef the		

^{*} The results of these tests are usually reported separately

Soils are classified and described according to their engineering properties and behaviour.

The soil of each stratum is described using the Unified Soil Classification System¹ modified slightly so that an inorganic clay of "medium plasticity" is recognized.

The modifying adjectives used to define the actual or estimated percentage range by weight of minor components are consistent with the Canadian Foundation Engineering Manual².

Relative Density and Consistency:

Cohensionless Soils		Cohesive Soils					
Relative Density	SPT (N) Value	Consistency	Undrained Shear Strength c _u (kPa)	Approximate SPT (N) Value			
Very Loose	0-4	Very Soft	0-12	0-2			
Loose	4-10	Soft	12-25	2-4			
Compact	10-30	Firm	25-50	4-8			
Dense	30-50	Stiff	50-100	8-15			
Very Dense	>50	Very Stiff	100-200	15-30			
*		Hard	>200	>30			

Standard Penetration Resistance ("N" value)

The number of blows by a 63.6kg hammer dropped 760mm to drive a 50 mm diameter open sampler attached to "A" drill rods for a distance of 300 mm after an initial penetration of 150 mm.

¹ "Unified Soil Classification System", Technical Memorandum 36-357 prepared by Waterways Experiment Station, Vicksburg, Mississippi, Corps Engineers of U.S. Army. Vol. 1 March 1953

² "Canadian Foundation Engineering Manual", 4th Edition, Canadian Geotechnical Society, 2006.

MODIFIED UNIFIED SOIL CLASSIFICATION SYSTEM INCLUDING IDENTIFICATION AND DESCRIPTIONS **LABORATORY GRAPH GROUP** MAJOR DIVISION TYPICAL DESCRIPTION CLASSIFICATION SYMBOL SYMBOL **CRITERIA** WELL GRADED GRAVEL, GRAVELS -GW $C_U \ge 4$ GRAVELS MORE THAN HALF THE COARSE FRACTION LARGER THAN 4.75mm and $1 \le C_C \le 3$ LARGER THAN 75μm) **CLEAN GRAVEL** SAND MIXTURES, LITTLE OR NO FINES (TRACE OR NO FINES) POORLY GRADED GRAVELS, GRAVEL- SAND MIXTURES, LITTLE OR NO NOT MEETING ABOVE GP FINES REQUIREMENTS ATTERBERG LIMITS GM SILTY GRAVELS, GRAVEL-SAND-SILT MIXTURES BELOW "A" LINE OR P.I. LESS THAN 4 DIRTY GRAVFI SOIL (WITH SOME OR MORE FINES) ATTERBERG LIMITS ABOVE GRAINED GC CLAYEY GRAVELS. GRAVEL-SAND- CLAY MIXTURES "A" LINE P.I. MORE THAN 7 SW SANDS MORE THAN HALF THE COARSE FRACTION SMALLER THAN 4.75mm WELL GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES $C_U \ge 4$ $1 \le C_C \le 3$ and COARSE THAN HALF BY CLEAN SANDS (TRACE OR NO FINES) NOT MEETING ABOVE SP POORLY GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES REQUIREMENTS ATTERBERG LIMITS SMSILTY SANDS, SAND-SILT MIXTURES **DIRTY SANDS** BELOW "A" LINE OR (MORE P.I. LESS THAN 4 (WITH SOME OR MORE FINES) ATTERBERG LIMITS ABOVE SC CLAYEY SANDS, SAND-CLAY MIXTURES "A" LINE P.I. MORE THAN 7 HUMAN PLACED MATERIALS (FILL) SHOULD BE DESCRIBED FOR THEIR ENGINEERING LIQUID LIMIT SILTS BELOW "A" LINE NEGLIGIBLE ORGANIC CONTENT PROPERTIES IN ACCORDANCE WITH THE MODIFIED USCS STANDARDS AS IF THEY WERE **FILL** W_L(%) NATURALLY OCCURRING SOILS SOILS SMALLER THAN 75µm) INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY SANDS OF W_I< 50% ML 1. CLASSIFICATION IS BASED SLIGHT PLASTICITY UPON PLASTICITY CHART (SEE BELOW) INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS, FINE SANDS OR SILTY $W_1 > 50\%$ MH 2. ALL SIEVE SIZES ON THIS SOILS SHEET ARE U.S. STANDARD CLAYS ABOVE "A" LINE NEGLIGIBLE ORGANIC CONTENT (ASTM E11). INORGANIC CLAYS OF LOW PLASTICITY, GRAVELLY, SANDY OR SILTY W_I< 30% CL CLAYS, LEAN CLAYS 3. IF THE NATURE OF THE FINES FINE-GRAINED (MORE THAN HALF BY WEIGHT HAS NOT BEEN DETERMINED, IT IS DESIGNATED BY THE LETTER INORGANIC CLAYS OF MEDIUM PLASTICITY, SILTY CLAYS CI 30% <W₁< 50% "F",(e.g. SF IS A MIXTURE OF SAND WITH SILT OR CLAY). W₁> 50% INORGANIC CLAYS OF HIGH PLASTICITY, FAT CLAYS CH 4. COARSE GRAIN SOILS WITH 5% TO 10% FINES GIVEN ORGANIC SILTS & CLAYS BELOW "A" LINE COMBINED GROUP SYMBOLS, W_I< 50% E.G. GW-GP IS A WELL GRADED ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY OL OR POORLY GRADED GRAVEL SAND MIXTURE WITH CLAY BINDER BETWEEN 5% AND 10% W_L> 50% ORGANIC CLAYS OF HIGH PLASTICITY FINES. OHBEL STRONG COLOUR OR ODOR, AND HIGHLY ORGANIC SOILS PEAT AND OTHER HIGHLY ORGANIC SOILS OFTEN FIBROUS TEXTURE Defining Ranges of % by PLASTICITY CHART FOR SOILS PASSING 425 µm SIEVE SOIL GRAIN SIZE RANGE AND DISTRIBUTION Weight of Minor Components Material Fraction Sieve size (grain size) Approx. Scale Size PERCENT DESCRIPTION Boulders >(200-mm) Larger than a soccer ball 35 - 50AND 50 20 - 35 Y / EY , Retained on 3-in (75-mm) CH INDEX Cobbles Fist-size to Soccer ball 10 - 20 SOME sieve & < (200-mm) 40 1 - 10 TRACE Passes 3-in (75-mm) sieve & Coarse Thumb-sized to fist-sized retained on 3/4 -in (19-mm) sieve Gravel Passes 3/4 -in (19-mm) sieve & 30 Fine Pea-sized to Thumb-sized retained on No.4 (4.75-mm) sieve CI MH Passes No.4 (4.75-mm) sieve & Coarse Rock Salt to Pea-sized retained on No.10 (2.00-mm) sieve 20 Passes No.10 (2.00-mm) sieve &

retained on No.40 (425-µm) sieve

Passes No.40 (425-μm) sieve &

Passes No. 200 (75-µm) sieve

< 0.075mm

retained on No.200 (75-um) sieve

Medium

Fine

Sand

Fines

(Silt/Clay

Sugar-sized to Rock Salt

Flour sized to Sugar-sized

Flour-sized and smaller

ENVIRONMENT & INFRASTRUCTURE SOLUTIONS A DIVISION OF WOOD CANADA LIMITED

#600 - 4445 LOUGHEED HIGHWAY BURNABY, BC V5C 0E4 TEL. 604-294-3811 FAX 604-294-4664

PLASTICITY CL OH OL 10 ML CL - ML Actual Size 0 0 10 20 30 60 70 80 90 LIQUID LIMIT

MODIFIED UNIFIED SOIL CLASSIFICATION SYSTEM

DWN BY:	DATUM:	DATE:
HY	N/A	DECEMBER 2019
CHK'D BY:	REV. NO.:	PROJECT NO:
TM	N/A	NX14001C1.3
PROJECTION:	SCALE:	REFERENCE NO.:
N/A	AS SHOWN	GEO - 1110

CLIENT: Nyrstar Myra Falls Ltd.	F	PROJECT: Myra Falls						BOREHOLE	NO: BH19-01			
DRILLER/DRILL: Mudbay Drilling/Soni	c/Becker E	British Columbia, BC						PROJECT N	IO: NX14001C1.3			
METHOD: Sonic Drilling	N	NORTHIN	G: 549	94704 E	STING: 3	11709.9	9		ELEVATION	l: 365.1 m		
SAMPLE TYPE TUBE	✓ NO RECOV	/ERY	igwedgeSF	PT		GRAE	3		LPT	CORE		
BACKFILL TYPE BENTON	ITE BENTONITE	E CHIPS	BE	ENTONITE	PELLETS .	GRO	JT		PIEZOMETE	R HEAD SAND		
	SOIL CRIPTION	SAMPLE TYPE	SAMPLE NO		—SCPT 200 —CPT 200 "▼LPT "N" 図iB 80 PLASTIC N	IC L	IQUID			DITIONAL RMATION	BACKFILL	ELEVATION (m)
GRAVEL, sandy, some s Gravel is subangular t	b be placed in 0.61 m to 0.91	m				60		C2:	REC:1.22/1.52 m REC:1.52/1.53 m REC:2.9/3.04 m			364- 364- 363- 361- 360- 359- 358-
wood.	Environment & Infra 600 - 4445 Louç Burnaby, B	gheed H	ighw	/ay	ENTERED	BY: H	ΙΥ			ETION DEPTH: 3 ETION DATE: 11	/26/201	
ŏ <u> </u>			-		REVIEWE	וסע.	DVV				Page	1 01 4

CLIENT: Nyrstar Myra Falls Ltd.		PROJECT:	Myra Falls			BOREHOLE NO: BH19-01			
DRILLER/DRILL: Mudbay Drilling/Sor		British Colur					PROJECT NO: 1	NX14001C1.3	
METHOD: Sonic Drilling		NORTHING		ASTING: 31	11709.9		ELEVATION: 36		
SAMPLE TYPE TUBE	✓ NO RECO	VERY	SPT		GRAB	-	LPT	CORE	
BACKFILL TYPE BENTOI	NITE BENTONI	TE CHIPS	BENTONIT	E PELLETS			■ PIEZOMETER HE	AD SAND	
	SOIL CRIPTION	SAMPLE TYPE	ON 10 10 10 ASPT 4	—SCPT \ 00 200 —CPT q 00 200 "N"▼LPT "N" ▼IBP 0 80 PLASTIC MC	V _s (m/s) 300 400 (BAR) 300 400 T N(60) (BLOWS/300 r 120 160		ADDITIC INFORM <i>i</i>		ELEVATION (m)
111 112 113 114 115 115 116 117 118 118 119 119 119 119 119 119 119 119						C7:	REC:2.74/3.05 m REC:2.44/3.05 m		355- 355- 354- 353- 353- 351- 348- 348- 348-
E SOILS M	Environment & Inf	rastructure	Solution	s LOGGED E	BY: HY			DN DEPTH: 36.6 n	
Wood.	600 - 4445 LOI	ugheed Hig BC V5C 0E	nway	ENTERED	BY: HY		COMPLETIO	ON DATE: 11/26/2	
	Burnaby,	DC VOC UE	.4	REVIEWE	D BY: BW			Page	e 2 of 4

CLIENT: Nyrstar Myra Falls Ltd.		PROJEC	T: My	ra Falls					BOREHO	LE NO: BH19-01			
DRILLER/DRILL: Mudbay Drilling/Soni	c/Becker	British Co	olumbi	a, BC					PROJEC ⁻	PROJECT NO: NX14001C1.3			
METHOD: Sonic Drilling		NORTHII	NG: 5	494704 E	ASTING: 3	311709.9	9		ELEVATION	ON: 365.1 m			
SAMPLE TYPE TUBE	✓ NO RECC	VERY	\boxtimes s	SPT		GRA	3		LPT	CORE			
BACKFILL TYPE BENTON	ITE BENTONI	TE CHIPS	Ċ E	BENTONITI	PELLETS .		UT		■ PIEZOME	TER HEAD SAND			
	SOIL CRIPTION	SAMDIE TVDE	=	20 10 10 A SPT 40	—SCPT 200 —CPT 200 200 10"▼LPT"N" ▼IB IB 80 PLASTIC IN	Supermoided (kPa) 60 T V _s (m/s) 300 q _s (BAR) 300 BPT N(60) (BLi 120 MC 60	400 400 0WS/300 mm) 160 IQUID 100			DDITIONAL FORMATION	BACKFILL	ELEVATION (m)	
-21 -22 -23 -24 -25 -26 -26 -20 -21 -22 -23 -24 -25 -26 -26 -26 -26 -27 -28 -28 -29 -20 -20 -20 -20 -20 -20 -20 -20 -20 -20	sandy, some silt to silty, su	24.4m y,				•		Sc Cw to	10: REC: 2.59/3.05	m r to have smaller gravel size		344- 344- 341- 340- 333- 336-	
WARSE SOILS MYR WARSE SOILS MYR OF SOILS M	Environment & Inf	ugheed	High	olution way	ENTERED	DBY: H	ΙΥ			PLETION DEPTH: 3 PLETION DATE: 11		19	
Ö TTOO.	Burnaby,	BC V5C	0E4		REVIEWE	D BY:	BW				Page	3 of 4	

CLIENT	T: Nyrst	ar Myra Falls Ltd.		PROJEC	CT:	Myra Falls					BOREHOLE NO: BH19-01		
DRILLE	ER/DRIL	L: Mudbay Drilling/Soni	c/Becker	British C	Colun	nbia, BC					PROJECT NO: NX14001C1	.3	
METHO	DD: Sor	nic Drilling		NORTH	ING:	5494704 E	ASTING: 3	11709.	9		ELEVATION: 365.1 m		
SAMPL	E TYPE	TUBE	✓ NO REC	OVERY	\triangleright	SPT		GRAI	3	[::]	LPT COR	E	
BACKF	ILL TYF	PE BENTON	ITE BENTON	NITE CHIPS	S 🕻	BENTONIT	E PELLETS .	GRO	UT	I	PIEZOMETER HEAD [SAN)	
DEPTH (m)	SOIL SYMBOL		SOIL CRIPTION		SAMPLE TYPE	20	—SCPT 0 200 —CPT 0 200 "N"▼LPT "N" ■IBI 0 80 PLASTIC M	V _s (m/s) 300 q _s (BAR) 300 PT N(60) (BL 120	400 400 OWS/300 mm) 160 LIQUID 1 80		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-		damp to moist (colluvial,	ned, some fine gravel, tranative) sandy, some silt to silty, m	30.2m/						Lands	drilling inferred from 27 m to 30 m depth lide type materials or random backfill		335
-31 -32 -33		brown (colluvial, native) GRAVEL, fine to coarse,	some sand to sandy, som noist, brown (colluvial, nati	e silt, ve)						C12 F	R EC : 1.68/3.05 m		334
- - - - - - - - - -		GRAVEL, angular to submoist, grey (colluvial, nat	angular, some sand, some ive)	33.2m silt,						depth	50 mm of brown staining observed at 36	0 m	
-35		Occasional subrounded depth	d gravel observed below 3	3 m									331
-36 				36.6m									329
-37		cement bentonite grout Installed 85 mm Slope In Installed protective stick-	ed, wet) at 35.0 m depth 6.6 m to ground surface wi clinometer casing (3.34 ind up cover using bentonite c	ch OD)						installe - Insta was be concre - The prever	e Inclinometer casing, 3.34 inch (85 mm) ed in cement-bentonite grout mix. Illed well cover protection (Stick-Up type) ackfilled with bentonite chips only instead te for easy removal. protective cover must be removed carefu- ting damge to the casings prior or during am raise.	which of	328
-38 		easy removal for next rais	se										327
-39 													
			Environment & In	frastruc	ture	Solution	LOGGED	BY: H	<u> </u>		COMPLETION DEPTH:		
	W	ood.	600 - 4445 Lo Burnaby	, BC V50	C OE	iiway 4	REVIEWE				COMPLETION DATE:		19 4 of
L			1				I \	וט ט.	₽ ¥ ¥			ı aye	7 01

CLIENT: Nyrstar Myra Falls Ltd.	PROJECT: Myra Fal	ls	BOI	REHOLE NO: BH19-02		
DRILLER/DRILL: Mudbay Drilling/Sonic/Becker	British Columbia, BC			OJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling		5.6 EASTING: 311729.7		EVATION: 362.4 m		
_	RECOVERY SPT	GRAB	LPT			
BACKFILL TYPE BENTONITE BEI SOIL DESCRIPTION	SAMPLE TYPE SAMPLE NO	NITE PELLETS GROUT 20 S _{JPant} O S _{JPantaloto} (kPa) 80 80 100 200 300 400 −CPT q (BAR) 100 200 300 400 SPT'N"▼ LPT'N" IBJEPT N(60) (BLOWIS/300 mm) 40 80 120 160 PLASTIC MC LIQUID	PIE	ZOMETER HEAD SAND ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
GRAVEL, fine to coarse, angular to subangular trace to some silt, dry to damp, grey (waste round of the coarse). -1 -1 -2 -3 -4 -4 -4 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5 -5	k fill)	2049 40 60 80 51 64 47 40 48 55 66 66 66 66 66 66 66 66 66 66 66 66	as-built loca	surveyed borehole location had to be order to be within 2.0 m from the iBPT tion. cated at 1.5 m from iBPT19-02.		362
The survey of the state of the	th & Infrastructure Soluti	101 1120 1120 1120 1128 124 43 43 43 47 66 78 122 101	C3: REC:2.4		6 m	357—356—355—355—353—353—353—353—353—353—353
Wood. Environment 600 - 444	& Infrastructure Soluti 5 Lougheed Highway	ENTERED BY: HY		COMPLETION DATE: 11/2		9
Burn	by, BC V5C 0E4	REVIEWED BY: BW				1 of 4

CLIENT: Nyrstar Myra Falls Ltd.	Pi	ROJECT:	Myra Falls			В	OREHOLE NO: BH19-02		
DRILLER/DRILL: Mudbay Drilling/Sonic		ritish Colu				PF	ROJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling	N		G: 5494655.6			El	_EVATION: 362.4 m		
SAMPLE TYPE TUBE	✓ NO RECOVE		SPT		GRAB	LF			
BACKFILL TYPE BENTONI	TE BENTONITE	CHIPS	BENTONITI	E PELLETS	•	■ PI	EZOMETER HEAD SAND		
	SOIL RIPTION	SAMPLE TYPE	ON 10 10 SPT 40 40 40 40 40 40 40 40 40 40 40 40 40	— SCPT V _s 0 200 — CPT q, (i) 0 200 "N"▼ LPT "N" 図 iBPT 0 80 PLASTIC MC	(m/s) 300 400		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
SAND, fine to coarse grain (fill) GRAVEL, angular to suba sandy, trace to some silt, the each lift (waste rock fill) Gravel is fine to coarse	ned, gravelly, silty, moist, browning and silty, moist, browning and silty a	13.1m wn	20	555 49 88 88 78 78	60 so 1 <u>52</u> 1 <u>52</u> 1 <u>4</u> 5	C7: REC-2	2.59/3.05 m		352- 351- 350- 349- 347- 345- 344-
37 00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					1 <u>2</u> 5 140				
SO SO	Environment 0 led		ro Coluiti-	I OGGED B			COMPLETION DEPTH: 3	9 6 m	
Wood.	Environment & Infra 600 - 4445 Loug	jheed Hi	ighway	ENTERED I	BY: HY		COMPLETION DATE: 11		19
§ ₩ 000.	Burnaby, BC	C V5C 0	Ĕ4	REVIEWED			25 22011 57.112. 11	Page	

CLIENT:	Nyrsta	r Myra Falls Ltd.		F	ROJEC	T: M	ra Falls					BOR	EHOLE NO: E	3H19-02		
DRILLER	/DRILL	: Mudbay Drilling/Son	ic/Becker	E	British Co	lumb	ia, BC					PRO	PROJECT NO: NX14001C1.3			
		c/iBPT Drilling						EASTING:		.7		ELE,	VATION: 362			
SAMPLE		TUBE		NO RECOV					GRAB			LPT		CORE		
DEPTH (m) CORE RUN	7		SOIL CRIPTION	BENTONITI	= CHIPS	I ON II	10 10 A SPT 4	—SCPT 200 —CPT 0 0 200 "N"▼LPT "N" 図iBf 0 80 PLASTIC M	Usemolded (kPa) 60 Vs (m/s) 300 qs (BAR) 300 PT N(60) (BLO) 120	80 400 400		PIEZ	OMETER HEAI ADDITION INFORMAT	IAL	BACKFILL	ELEVATION (m)
		Hard drilling from 23.8	to 24.4 m depth				: :	68	121 121	1 80	191-	8: REC:2.9/3				342- 341- 340- 339- 338-
COARSE SOILS MYRA BH-2019.GPJ 1/31/20 54 54 54 54 54 54 54 54 54 54 54 54 54		One piece of organic GRAVEL, fine to mediun ine to coarse grained, s ock fill) GRAVEL, fine to mediun race to some silt, damp	n, rounded to sub ome to silty, mois n, subangular to a grey (waste rock	orounded, sar st, grey (waste angular, sand k fill)	29.6m y,	LPT	2				c	10: REC:1.5 11: REC:1.5 PT2: Blow C EC:0.46/0.44	2/1.52 m >ount 62-81-88 6 m			335- 334- 333-
E SO			Environm	ent & Infra	astruct	ure S	Solution	SLOGGED	BY: HY				COMPLETION			
DARS	W	ood.	600 -	4445 Loug Surnaby, B	gheed l	ligh	way	ENTERED	BY: H			-	COMPLETION			
8			Ь	arriaby, D	- VJC	J_4		REVIEWE	ηΒΑ: Ε	VV				P	age 🤅	3 of 4

CLIEN	IENT: Nyrstar Myra Falls Ltd.						PROJECT: Myra Falls								ВС	BOREHOLE NO: BH19-02			
DRILLE	ER/DF	RILL:	Mudbay Drilling/Soni	c/Becker		British Columbia, BC								PF	PROJECT NO: NX14001C1.3				
METHO	DD: S	Sonic/i	BPT Drilling						94655.6	EASTI	NG: 31	11729.7	'		ELEVATION: 362.4 m				
SAMPL	E TY	PΕ	TUBE		NO RECC	VERY		\boxtimes s				GRAB			[::]LP		С	ORE	
BACKF	ILL T	YPE	BENTON	ITE	BENTONI	TE CHIF	PS	В	ENTONITI	PELLE	TS.	GROUT			■ PII	ZOMETER	R HEAD S	AND	
DEPTH (m)	CORE RUN	SOIL STIMBOL		SOIL CRIPTIO	N		SAMPLE TYPE	SAMPLE NO	20 10 10 ▲ SPT 40	0 2 0 2 "N" V LPT "N 0 8 PLASTIC	Peak O SuRemoid O SUREMOID SCPT V _s (i O0 CPT Q _s (Bs O0 " ■ iBPT N O0 MC	n/s) 300	160				OITIONAL RMATION	BACKFILL	ELEVATION (m)
- - - - - - - 31		્રૅ∢ silt	RAVEL, fine to medium ty with trace of clays, lo ange mottled (till-like m	w plasticity, m	noist, grey wit	30.5m ned, h		LPT3						100	EC :0.3/0.	v Count 85-10 46 m 1.37/1.52 m	10+	1	332
-32 c			RAVEL, subrounded to ey (colluvial, native)	subangular, s	andy, silty, da	32.6m amp,		LPT4						: ′∯۱	REC:0.46/	v Count 58-50 J.46 m 1.52/1.50 m	1-70		• "
- - - - - -34	14 0 00 000	્રૅ∢ silt	RAVEL, fine to medium ty with trace of clays, lo ange mottled (till-like m	w plasticity, m	noist, grey wit		_							(C14: REC:	1.37/3.05 m			
-35																			328
-36 	15 S	sol ler	RAVEL, subrounded to me silt, moist, grey (col is of brown staining at	lluvial, native) 36.8 m depth	_	37.2m									C15: REC:	2.9/3.04 m			. 320
- - - - -38		tra	ND, fine to coarse grai ice silt, moist, grey (coll RAVEL, rounded to sub me silt to silty, moist, g	uvial, native) rounded up to	100 mm ø, s	37.5m	-												325
COARSE SOILS MYRA BH-2019,GPJ 1/31/20		GF oliv	RAVEL, subrounded to ve grey (till-like materia	subangular, s ls, native)	andy, silty, m	39.0m oist, 39.6m								E	Borehole g sement be nstalled V nitial read nitial read 2/1/2019	ntonite grout W63327 at ele ing prior install ings after insta at 9384.8B, 11	9.6 m to ground suvation 331.9 m ation: 9381.3B, 10 Iltion: 1.3°C = 332.4 m		322
				Envisor	ment & Inf	roct		rc C	alu4ia	LOGG	ED RV	′ HY	:	: 1	2/6/2019		3°C = 322.2 m ETION DEPT	H: 39.6 m	
ARSE	W	/ 0	od.	600	- 4445 Lo	ughee	d H	ighv	ay /ay	ENTE	RED B	Y: HY					ETION DATE		
<u> </u>	4 1				Burnaby,	BC V5	C 0)E4		REVIE	EWED	BY: BV	٧					Page	4 of

CLIENT: Nyrstar Myra Falls Ltd.	PROJEC	CT: Myra Falls		BC	REHOLE NO: BH19-03		
DRILLER/DRILL: Mudbay Drilling/Sonio	c/Becker British C	Columbia, BC		PR	ROJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling			EASTING: 311805.5	EL	EVATION: 362.4 m		
SAMPLE TYPE TUBE	NO RECOVERY	SPT	GRAB	LP'			
BACKFILL TYPE BENTONI	ITE BENTONITE CHIPS	S BENTONITE	PELLETS GROUT	PIE	EZOMETER HEAD SAND		
)	v ,	◆ S _{unate} O S _{ulterelosed} (kPa) 40 60 80 80 80 80 80 80 80 80 80 80 80 80 80		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
GRAVEL, fine to coarse, fine to coarse, sine to coarse, sine to coarse grained, so grey/brown (waste rock file) -1 -1 -2 -3 -3 -4 -4 -45 -57788788788788787887887887887887888	angular to subangular,			C1: REC.2 BH19-03 C2: REC.3. C3: REC.2.	05/3.05 m		362
wood.	Environment & Infrastruct 600 - 4445 Lougheed Burnaby, BC V5C	d Highway	LOGGED BY: HY ENTERED BY: HY REVIEWED BY: BW		COMPLETION DEPTH: 36 COMPLETION DATE: 12/	1/2019	1 of 4

CLIEN	CLIENT: Nyrstar Myra Falls Ltd.							: Myra Falls		ВО	BOREHOLE NO: BH19-03				
			L: Mudbay Drilling/Soni	c/Becker		British Columbia, BC							PROJECT NO: NX14001C1.3		
			nic/iBPT Drilling						EASTIN	IG: 311805.5		ELI	EVATION: 362.4 m		
SAMP	LE	TYPE	_		NO RECO			SPT		GRAB		LP	L		
DEPTH (m)	CORE RUN	占		SOIL CRIPTI	BENTON	ITE CHIF	SAMPLE TYPE	ON 10	0 40 00 20 00 20 00 20 00 20 00 80 PLASTIC	SCPT V _s (m/s) 300 4 CPT q. (BAR) 300 4 300 4 300 4 300 120 1 MC LIQUID	00 00 00 00 00 00 00	PIE	ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
	C5-		SAND, fine to coarse, silt (waste rock fill) GRAVEL, fine to coarse, occasional cobbles, sand brown/orange/grey (waste	angular to	subangular,	12.2m e/grey 12.5m		23 23 23 26	57 9			:5: REC:2.4	44/3.05 m		352 351 350
	C6-		SAND, fine to coarse grain brown/grey (waste rock fi	ined, grave ll)	lly, silty, moist,	15.2m		21 18 31 29 25 18			c	66: REC:3.0	05/3.05 m		348 347
			GRAVEL, fine to coarse, occasional cobbles, some brown/orange/grey (waste	e sand, trac		16.8m			58 57 57 57 44						346
- - - - - - - - - - - - - - - - - - -	C7-		GRAVEL, fine to coarse gandy, fine to coarse grain (waste rock fill) GRAVEL, fine to coarse gailty, some fine sand, moi	ned, some	silt, moist, brow	n 19.4m		25 25 27 27	44 65 48			:7: REC:3.0	05/3.05 m od fragments found here		344
-	\	W	ood.	Enviro 60	onment & Inf 00 - 4445 Lo Burnaby,	ughee	d H	re Solution ighway	ENTER	ED BY: HY RED BY: HY WED BY: BW			COMPLETION DEPT		

-	ENT: Nyrstar Myra Falls Ltd. LLER/DRILL: Mudbay Drilling/Sonic/Becker								yra Falls					В	OREHOLE	NO: BH19-0	3	
					cker		British Co							_		O: NX14001	C1.3	
-			ic/iBPT Drillin	<u> </u>					5494650.7	EASTIN				=	LEVATION			
SAMF				TUBE		RECOV					GR					T c		
BACK (m) HZGO	CORE RUN	ار ا	<u> </u>	SOI DESCRI	L	ENTONITE	1021	N 8		0 40 0 200 00 200 00 200 00 200 "N"▼ LPT "N" 0 80 PLASTIC	CPT Q _s (BAR) 300 CPT Q _s (BAR) 300 EPT Q _s (BAR) 300 MC	80 400 400 (BLOWS/300 mm 160 LIQUID	1)	PI		RHEAD [:]S. DITIONAL RMATION	AND RACKELL	ELEVATION (m)
-21	C8-		GRAVEL , fine some sand to grey/brown (w	to coarse graine sandy, trace to s aste rock fill)	ed, angular to s ome silt, damp	subangula o to dry,	21.9m ır,		2 15	56 56 76 78	60	80	c	8: REC:	3.05/3.04 m			342
-23 -24 25	C9-						\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	LPT	1				R N	PT1: Blo EC:0.43 asteroc	k lifts appear to	0-53 be placed <0.3 m seen clearly (6 lift	thickness s/1.5m)	339
-26	C10-							— LPT	2				: LI	/asterocl s various 10: REC PT2: Bld EC:0/0.4	w Count85-60	be placed > 0.3 n seen clearly (4 lift	n thickness s/ 1.5 m)	337 336
28	C11+							LPT	3				17 & R	11: REC PT3: Bld EC:0.04	:1.52/1.53 m w Count 170/1 0.46 m	.5"		335
COARSE SOILS MYRA BH-2019.GPJ	C12-	_		to coarse graine some silt, damp	o to dry, grey/b	rown (wa	ste	LPT					85¢ L R	12: REC PT4: Blo EC:0.25				333
SE SC	_			Er	nvironment	& Infra	astruct	ure S	Solution	SLOGGE	D BY: I	HY			_	ETION DEPT		
OARS	1	W(J.	600 - 444	45 Louตุ าaby, B	gheed	High	way	ENTER	ED BY:	HY			COMPLI	ETION DATE		
ŏ	wood.				2011	٠				REVIE	∧⊏∩ R.J	. RM					Page	3 of

CLIE	NT:	Nyrst	ar Myra Falls Ltd.		PROJE	СТ	: Myra Fa	alls						BO	OREHOLE NO: BH19-03		
DRILI	LER	/DRIL	L: Mudbay Drilling/Son	ic/Becker	British (Col	umbia, B0	С						PF	ROJECT NO: NX14001C1.3		
METH	HOD): Sor	nic/iBPT Drilling		NORTH	HIN	G: 54946	550.7	EASTIN	IG: 3	11805	5.5		EL	_EVATION: 362.4 m		
SAM	PLE	TYPE	TUBE	✓ NO REC	OVERY		SPT				GRAE	3		LP	PT CORE		
BACK	(FIL	L TYF	PE BENTON	IITE BENTON	IITE CHIP	S	BENT	ONITE	PELLET	S.	GRO	JT		■ PII	EZOMETER HEAD SAND		
DEPTH (m)	CORE RUN	SOIL SYMBOL		SOIL CRIPTION		SAMPLE TYPE	SAMPLE NO	40	20	CPT q; (B	m/s) 300 AR) 300 N(60) (BL0	400 400 20WS/300 mm 160 1QUID 180	1)		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-31	C13-	_	becomes moist below GRAVEL, fine to medium	ger size, at 30.2 m depth 30.3 m depth n, rounded to subrounded, odamp, grey (colluvial, na			LPT5						12	when adde C13: REC:	1.52/1.52 m w Count55-60/3"		332
-32 32	C14-		GRAVEL, sandy (fine to moist, grey/brown (colluv	coarse grained), some silt i ial, native)	32.0m to silty, 32.6m		LPT6						9		1.52/1.53 m w Count 45-43-55 0.45 m		330-
-33	C15-		silty with trace of clays, lo orange mottled (till-like m GRAVEL, silty, trace to s (colluvial, native) GRAVEL, subangular, oc trace silt to some silt, dar native) GRAVEL, subrounded to	ome sand, dry to damp, groccasional cobbles, some samp to moist, grey (till-like manning subangular, occasional coamp, grey (colluvial, native	th 33.2m ey 33.5m ind, naterials, 34.1m bbles,									C15: REC:	:1.52/3.05 m		329- 328-
-35 -36 -37	C16-			grained, rounded to subrou ly, trace silt to some silt, m										C16: REC:	3.05/3.04 m		327
2019.GPJ 1/31/20				ı grained, rounded, occasic ne silt to silty, moist, grey (Borehole g cement be Installed V Initial read water used			325
COARSE SOILS MYRA BH-2019.GPJ			native)		39.6m				LOCC	ED DV	/· LI\	,		12/3/2019	ings after installtion: at 9662.1B, 11.7°C => 329.7 m at 9664.3B, 11.3°C => 329.6 m	Q m	323
3SE	•		224	Environment & In 600 - 4445 Lo	trastruc ougheec	ctu d H	re Solu ighwav	tions	ENTER	SED E	ı. ⊓1 }Y∙ H	Y			COMPLETION DATE: 12/)
JOAE	1	VV	000 .	Burnaby,					REVIE							Page	
\cup	wood.			1								•			1	٠ ت ٠	

CLIENT: Nyrstar Myra Falls Ltd.	PROJEC [*]	Т: Муг	ra Falls	ВС	REHOLE NO: BH19-04		
DRILLER/DRILL: Mudbay Drilling/Sonic/Becker	British Co	lumbia	a, BC	PR	OJECT NO: NX14001C1.3		
METHOD: Sonic Drilling	NORTHIN	NG: 54	194669 EASTING: 311809.2	EL	EVATION: 365.1 m		
SAMPLE TYPE TUBE NO REC	COVERY	\boxtimes s	PT G RAB	LP	T CORE		
BACKFILL TYPE BENTONITE BENTO	NITE CHIPS	ζ	ENTONITE PELLETS GROUT	■ PIE	ZOMETER HEAD SAND		
SOIL DESCRIPTION	SAMPI F TYPE		S _{inhead} O S _{identidate} (kPa) 40 60 80 -SCPTV ₂ (mis) 100 200 300 400 -CPT q ₁ (BAR) 100 200 300 400 SPT "N" ▼ LPT "N" ZiBPT N(60) (BLOWS/300 mm) 40 80 120 160 PLASTIC MC LÍQUID 20 40 60 80		ADDITIONAL INFORMATION	BACKFILL	
NO SAMPLING Research Resea	<u>45.7m</u>		olution _{\$} LOGGED BY: HY	END OF BOM STATE OF THE PROPERTY OF THE PROPER	ampling was required, and sonic casing ced without recovery. DREHOLE AT 45.7m water encountered. The state of the state o		
WOOD. Environment & II 600 - 4445 L Burnaby	ıırastructi	ure S	olutions - Coch bi. III		00mm LE 110M DEI 111. 40.	- 111	
600 - 4445 L	ougheed I , BC V5C	Highv	vay ENTERED BY: HY		COMPLETION DATE: 11/2	7/201	19

CLIENT: Nyrstar Myra Falls Ltd.	PROJECT	: Му	a Falls		BOREHOLE NO: BH19-05		
DRILLER/DRILL: Mudbay Drilling/Sonic/Becker	British Col				PROJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling			94664.1 EASTING: 311880.9		ELEVATION: 362.5 m		
	COVERY	\boxtimes s			LPT CORE		
BACKFILL TYPE BENTONITE BENTO	NITE CHIPS	В	ENTONITE PELLETS GROUT	[PIEZOMETER HEAD SAND		
SOIL SYMBOL SYMBOL SYMBOL SYMBOL	SAMPLE TYPE	SAMPLE NO	● S _{dract} O S _{demonister} (KPa) 40 60 80 80 100 200 300 400 100 200 300 400 100 200 300 400 100 200 300 400 100 200 300 400 100 100 80 120 160 100 PLASTIC MC LIQUID	im)	ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
GRAVEL, fine to coarse, angular to subangular, occasional cobbles, some sand to sandy, fine to co	parse		16 20 50 40 60 80	C1	: REC :3.05/3.05 m	W	
grained, trace to some silt, damp to moist, grey/bro (waste rock fill)	own		50 42 42 51 52 52 61 70 43 40 50 50 52	BH - B	BPT19-05B was completed 5 m east from PT19-05, which located 2.8 m north of proposed 19-05. But 19-05 was relocated to be at 1.5 m from PT19-05.		362 361 360 359
-5 less gravel, more silt from 5.8 m to 6.1 m depth			36 44 32	C3	9: REC :2.44/3.04 m		357 356
2 600 - 4445 L	ougheed H	lighv	합 달 달 설 13 plutions LOGGED BY: HY vay ENTERED BY: HY	C4	COMPLETION DEPTH: 39	2/2019	
Burnaby	, BC V5C (v ⊑ 4	REVIEWED BY: BW			⊃age	1 of

CLIENT: Nyrstar Myra Falls Ltd.	PROJEC	T: M	yra Falls		BOREHOLE NO: BH19-05		
DRILLER/DRILL: Mudbay Drilling/Sonic/Becker	British Co				PROJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling	NORTHII		5494664.1 EASTING: 311880.9		ELEVATION: 362.5 m		
SAMPLE TYPE TUBE	NO RECOVERY	\boxtimes		[::]	LPT CORE		
BACKFILL TYPE BENTONITE	BENTONITE CHIPS	\Diamond	BENTONITE PELLETS GROUT	I	PIEZOMETER HEAD SAND		
SOIL SYMBOL SYMBOL SYMBOL SYMBOL	CAMIDI E TVDE	SAMPLE I YPE			ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-12 -13 -14 -15 -16 -16 -17 -18 -17 -18 -18 -19 -19 -19 -19 -10 -10 -10 -10 -10 -10 -10 -10 -10 -10	15.5 m to 16.8 m		29 33 34 34 35 34 32 32 32 32 32 32 32 32 32 32 32 32 32	C6: RI	EC:2.29/3.05 m EC:2.44/3.05 m		352— 351— 349— 344— 344— 344—
Wood. Environme	ent & Infrastruct 1445 Lougheed urnaby, BC V5C	High	Solutions LOGGED BY: HY way ENTERED BY: HY REVIEWED BY: BW		COMPLETION DATE: 12		

CLIEN	IT: Ny	rstar N	lyra Falls Ltd.			PROJE(CT: N	1yra Fa	lls					BOF	REHOLE NO: BH1	9-05		
DRILL	.ER/DF	RILL: 1	Mudbay Drilling/Soni	c/Becker		British C	oluml	bia, BC						PRO	DJECT NO: NX14	001C1.3		
METH	IOD: S	Sonic/il	3PT Drilling			NORTH	ING:	549466	64.1 E	ASTING:	31188	0.9		ELE	:VATION: 362.5 m	1		
SAMF	LE TY	'PΕ	TUBE		NO RECC	VERY	\boxtimes	SPT			GRA	В	[:	LPT		CORE		
BACK	FILL T	YPE	BENTON	ITE	BENTONI	TE CHIPS	5 (X	BENTO	NITE P	ELLETS[GRO	UT	I	PIEZ	OMETER HEAD	SAND		
DEPTH (m)	CORE RUN	SOIL STMBOL		SOIL CRIPTIC	ON		SAMPLE TYPE		40	— SCP 200 — CPT 200 LPT "N" ■i6 80	120	400 400 .OWS/300 mm) 160 LIQUID			ADDITIONAL INFORMATION	V	BACKFILL	ELEVATION (m)
-21 -22 -23 -24			Very hard drilling from	22.9 to 24.1	m depth		LP	18 18 11 6 6 13							5/3.04 m 2/1.53 m Count 55-46-68 16 m			342
26	C10- 00 0 7 7	GR	AVEL, fine to medium	, angular to s	subangular, sor	26.4m	LP	Т2					100 ^{C10} PT2 REC	REC:1.5 2: Blow (:0.16/0.4	52/1.52 m Count 31-25/0.25" 16 m			336
		sar roc	nd to sandy, some silt, k fill)	moist, brown	n/orange/grey (waste 27.4m												- - - - -
-28		∑ sor	AVEL, fine to coarse g ne sand to sandy, trac wn/grey (waste rock fil	e to some silf	ılar to subangu t, damp to dry,	lar,	LP'	13					92LPT3 REC	REC:1.5 3: Blow (:0.08/0.4	52/1.53 m Count 32-42-50 46 m			335-
H-2019.GPJ 1/31/20	C12	∑ sar	AVEL, fine to medium to sandy, trace to so	, angular to s	subangular, sor st, brown/orang	29.0m ne le/grey	LP	T4					14612 148PT4 REC	REC:1.5 l: Blow (:0.46/0.4	52/1.52 m Count 38-85-53 15 m			334
COARSE SOILS MYRA BH-2019.GPJ		(wa	aste rock fill)															333
SES	X A	/-	لم	Environ	ment & Inf	rastruc	ture	Soluti	ions L	OGGED	BY: H	Y			COMPLETION DE			
OAR	M	/ O	od.	600) - 4445 Loı Burnaby,	agneed BC V50	nigi 0E4	ıway L		NTEREI EVIEWE					COMPLETION DA		2019 age 3	of 1
ŏL					, ,		. – .			ı⊏vı⊏VVE	וס ט.	אאם				78	aye s	UI 4

CLIEN	NT:	Nyrst	ar Myra Falls Ltd.		PROJ	ECT	: Myr	a Falls						В	BOREHOLE NO: BH19-05		
DRILL	ER	/DRIL	L: Mudbay Drilling/Soni	c/Becker	British									P	PROJECT NO: NX14001C1	1.3	
METH	IOD	: Sor	nic/iBPT Drilling		NORT			94664.1	I EAST					E	ELEVATION: 362.5 m		
SAMP	LE	TYPE	TUBE	✓ NO R	ECOVERY		\boxtimes s	PT			GRA	ιB		[#] L		RE	
BACK	FILI	L TYF	PE BENTON	ITE BENT	ONITE CHI	PS	В	ENTONIT						I P	PIEZOMETER HEAD SAN	ID .	
DEPTH (m)	CORE RUN	SOIL SYMBOL		SOIL CRIPTION		SAMPLE TYPE	SAMPLE NO	10 ▲ SPT	00 00 "N"▼LPT IO PLASTIC	S _{uPosk} ○ S _{uR} 40 -SCPT V 200 -CPT Q 200 "N" ▼IBPT 80 MC	/ _s (m/s) 300 (BAR) 300 T N(60) (BI 120	400	nm)	-	ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-31	C13-		two silty zones observed epth	ed between 30.5 m to 3	3.5 m		LPT5						1	\$ C13: REC \$ PPT5: BI REC:0.34	C:1.52/3.05 m ow Count51-43-85 4/0.46 m		332- 331-
-32							LPT6						1	PT6: BI REC:0.3/	l ow Count 27-32-69 0.46 m		330-
-34	C14-		SAND, fine to coarse grain (till-like materials, native) GRAVEL, some sand to so (colluvial, native)		33.5m	1								C14: REC	C :1.68/3.05 m		329-
-35					36.0m									-			328 ⁻
-36 	C15-		SAND, fine to coarse graidamp, grey (colluvial, nated and gray) GRAVEL, fine to coarse gray some sand, some silt, more sand, some sa	ive) grained, angular to suba sist, grey (colluvial, nativ	36.6m angular, ve)	1								C15: REC	C :3.05/3.04 m		326-
- - - - - - 38			GRAVEL, subrounded to moist, brown (colluvial, na	subangular, some sand ative)	d, silty,												325-
019.GPJ 1/31/20														Drillina	terminated at 39.6 m depth		324-
COARSE SOILS MYRA BH-2019.GPJ		000			39.6n									- No grou	undwater encountered le backfilled with cement-bentonite gr mpletuion.	out mix	323-
E SC	_		<u> </u>	Environment &	Infrastru	ıctu	ire S	olution	s LOG	GED E	3Y: H	Υ			COMPLETION DEPTH:		
DARS	1	M	ood.	600 - 4445	Loughee by, BC V	ed H	lighv	<i>l</i> ay	ENT	ERED	BY: I	HY			COMPLETION DATE:		
SI				Durilai	.,, DO V	(KEV	IEWED	J RA:	RM				Page 4	4 of 4

CLIENT: Nyrstar Myra Falls Ltd.	PRO	JECT:	Myra Falls			E	BOREHOLE NO: BH1905B		
DRILLER/DRILL: ConeTec/Becker	Britis	sh Colun	nbia, BC			F	PROJECT NO: NX14001C1.3		
METHOD: iBPT	NOR	RTHING:	5494664.1	EASTING: 31°	1875.9	E	ELEVATION: 362.5 m		
SAMPLE TYPE TUBE	✓ NO RECOVERY	Y >	SPT	⊟ G			_PTCORE		
BACKFILL TYPE BENTO	NITE BENTONITE CH	HIPS	BENTONITE	PELLETS G		I F	PIEZOMETER HEAD SAND		
	SOIL CRIPTION	SAMPLE TYPE	ON 100 100 100 100 100 100 100 100 100 10	— CPT q. (BAF 200 3 N"▼ LPT "N" ■ BPT N(6 80 1 PLASTIC MC	s) 00 400 R) 00 400		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
COARSE SOILS MYRA BH-2019 GPJ 1/3/120 8 1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1				\$\frac{1}{4}\$	124	No paire location.	ed set of borehole avialbler for this iBPT		362- 361- 360- 359- 358- 356- 354- 353-
wood.	Environment & Infrastr 600 - 4445 Loughe Burnaby, BC V	ed Hig	ahway	LOGGED BY: ENTERED BY REVIEWED B	': HY		COMPLETION DEPTH: 2	/2/2019	9 1 of 3

CLIENT: Nyrstar Myra F	Falls Ltd.	PF	ROJECT:	Myra Falls			BOREHOLE NO: BH1905B		
DRILLER/DRILL: Cone	Tec/Becker		itish Colun				PROJECT NO: NX14001C1.3		
METHOD: iBPT		NO	ORTHING:	5494664.1	EASTING: 311875.9		ELEVATION: 362.5 m		
SAMPLE TYPE	TUBE	NO RECOVE		SPT	GRAB				
BACKFILL TYPE	BENTONITE	BENTONITE	CHIPS 🌘	BENTONITE	PELLETS GROUT	I	PIEZOMETER HEAD SAND		
DEPTH (m) CORE RUN SOIL SYMBOL	SOIL DESCRIPTIO	ON	SAMPLE TYPE	•	— SCPT V _s (m/s) 200 300 40 — CPT q _s (BAR)	0 0 mm) 0	ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-11				36 12 41					352- 351-
-12 13				28 36 32					350-
-14				35	62 66				349-
- - - -15					6 <u>5</u> 왕 양				348
- - - - - -16				4 3c 3c 3c 3c					347-
-17				33 33 34 34					346
- - - -18				23 E	5 2				345-
07/15/1				19 32 36					344-
COARSE SOLLS MIRA BH-2018 COARSE SOLLS MIRA					46				343-
) 	Enviror	nment & Infras	structure	Solution	LOGGED BY: HY	'	COMPLETION DEPTH: 2		
WOO	O. 600	0 - 4445 Lougl Burnaby, BC	heed Hid	ihwav	ENTERED BY: HY REVIEWED BY: BW		COMPLETION DATE: 11	/2/2019 Page	
اذ		, ,			INLVILWED DT. DW			raye	∠ UI

CLIEN	LIENT: Nyrstar Myra Falls Ltd.					T: My	ra Falls						BOR	EHOLE NO:	BH1905B		
		RILL: ConeTec/Bec	ker		British Co								_	JECT NO: N			
METH	HOD: iE	BPT			NORTHI			1 EAS	TING:	31187	5.9		ELE\	/ATION: 362			
SAMF	PLE TY	PE T	UBE	✓ NO RECC	VERY	\boxtimes s				GRAI			LPT		CORE		
BACK	(FILL T	YPE B	ENTONITE	BENTONI	TE CHIPS	ДВ	ENTON						■ PIEZ	OMETER HEA	D SAND		
DEPTH (m)	CORE RUN		SOIL ESCRIPTIO	ON	L GW V C	SAMPLE I PE SAMPLE NO		100	40 MC	/ _s (m/s) 300 (BAR) 300 T N(60) (BL 120	400 400 OWS/300 mm 160 LIQUID	1)		ADDITIOI INFORMA		BACKFILL	ELEVATION (m)
-21 -22 -23							26 14 11 16 12	<u> </u>	74		144	**					341- 340- 339-
24 																	338-
- - - - 26																	337-
-27																	336- 335-
28 1/31/20 2/8 29 29 29 29 29 29 29 29 29 29 29 29 29																	334-
COARSE SOILS MYRA BH-2019.GPJ			,														333-
SE SC	•	10 1	Enviror	ment & Inf	rastruct	ure S	olutio	ns LO	GED E	3Y: H	Y				N DEPTH: 22		
DAR!	W	100d.	600) - 4445 Lo Burnaby,	ugneea	High	way	EN	ERED	BY: F	ΙΥ		(COMPLETIO	N DATE: 11/2		
ŏL								KE	/IEWEI	λR.	ДVV				P	age	3 of 3

GRAVEL fine to coses, angular to subergular, sandy, graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to medium, angular to subergular, sandy, all graphylocon (vasials rock fill) GRAVEL fine to come delivery fill graphylocon (vasials rock fill) all graphylocon (vasials rock fill) all graphylocon (vasials rock fill)	CLIENT: Nyrstar Myra Falls Ltd.	PROJEC*	T: Myra Falls		ВО	REHOLE NO: BH19-06		
SAMPLE TYPE BRUTONITE BENTONITE BENTONITE CHIPS BENTONITE PELETS CARB LPT CONTENTS CARBONITE CHIPS CARBONITE C		ic/Becker British Co	olumbia, BC					
BACKFILL TYPE BENTONITE BENTONITE CHIPS SOIL DESCRIPTION BENTONITE CHIPS SOIL	METHOD: Sonic/iBPT Drilling				ELE			
SOIL DESCRIPTION DESCRIP								
SOIL DESCRIPTION BEST SOIL DESCRIPTION GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste rock iii) GRAVEL, fine to coarne, angular to subanquiar, sandy, gray-former (waste	BACKFILL TYPE BENTON	IITE BENTONITE CHIPS	BENTONIT		_ PIE	ZOMETER HEAD SAND		1
GRAVEL fine to coarse, angular to submigular, sandy, into to coarse grands, some six to saily, dry to damp. 1	DEPTH (m) CORE RUN SOIL SYMBOL	SOIL ENERGY ENER	SAMPLE NO 10 ASPT 41	0 40 60 80 SCPT V _s (m/s) 0 200 300 400 CPT q (BAR) 0 200 300 400 'N'▼LPT 'N' 2 BIBPT N(60) (BLOWS/300 mm) 80 120 160 PLASTIC MC LIQUID)		BACKFILL	ELEVATION (m)
Environment & Infrastructure Solutions LOGGED BY: HY 600 - 4445 Lougheed Highway Burnaby, BC V5C 0E4 Environment & Infrastructure Solutions LOGGED BY: HY COMPLETION DEPTH: 39.6 m ENTERED BY: HY COMPLETION DATE: 12/3/2019 REVIEWED BY: BW	GRAVEL, fine to coarse, fine to coarse, fine to coarse, fine to coarse grained, st grey/brown (waste rock fine to coarse), fine to coarse, fin	angular to subangular, sandy, ome silt to silty, dry to damp, sill) 9.1m angular to subangular, sandy, sa		47 40 60 80 49 60 80 49 60 80 49 60 80 49 60 80 60 80 61 60 80 62 60 80 63 60 80 64 60 80 65 60	C2: REC:3.0	95/3.05 m		362— 361— 359— 358— 357— 355—
600 - 4445 Lougheed Highway Burnaby, BC V5C 0E4 Environment & intrastructure Solutions Logold B1: 111 COMPLETION DEPTH: 39.0 III COMPLETION DATE: 12/3/2019 REVIEWED RV: RW Page 1 of	Souls MYR	Employee and O. C. C.	C-1- (*)			COMPLETION DEDTH: 20	16 m	-
Burnaby, BC V5C 0E4 REVIEWED RV. RW Dage 1 of		Environment & Infrastructi 600 - 4445 Lougheed I	ure Solution Highway	ENTERED BA: HA)
TI TO THE PROPERTY OF THE PROP	MUUU.	Burnaby, BC V5C	0E4	REVIEWED BY: BW				

CLIENT: Nyrstar Myra Falls Ltd.		PROJECT:	Myra Falls			BOF	REHOLE NO: BH19-06		
DRILLER/DRILL: Mudbay Drilling/Son		British Colu					DJECT NO: NX14001C	1.3	
METHOD: Sonic/iBPT Drilling				EASTING: 3119			EVATION: 362.4 m		
SAMPLE TYPE TUBE	☑ NO RECO	<u>-</u>	SPT	GF		LPT			
BACKFILL TYPE BENTON	IITE BENTONI	TE CHIPS	BENTONITI	E PELLETS GF		L PIEZ	ZOMETER HEAD SAI	ND	
	SOIL CRIPTION	SAMPLE TYPE	SAMPLE NO 20 10 10 10 10 10 10 10 10 10 10 10 10 10	-SCPT V _s (m/s) 0 200 300 -CPT Q _s (BAR) 0 200 300 "N"▼LPT "N" ■BPT N(60) 0 80 120 PLASTIC MC	0 400 0 400 0 (BLOWS/300 mm) 0 160 LIQUID		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
111	to 14.9 m depth	19.8m	4	65 66 66 62 83 83 72 72 47 60 49		C5: REC:2.4:	3/3.05 m		352- 351- 350- 349- 345- 344- 343-
S B B B B B B B B B B B B B B B B B B B	Environment & Inf	rastructu	re Solution	LOGGED BY:	HY		COMPLETION DEPTH		
Wood.	600 - 4445 Lou Burnaby, I	iaheed Hi	iahwav	ENTERED BY:	: HY		COMPLETION DATE:		
8	Бигпару,	PC A2C 0	L4	REVIEWED BY	Y: BW			Page 2	2 of 4

CLIENT: Nyrstar Myra Falls Ltd.	CLIENT: Nyrstar Myra Falls Ltd.			PROJECT: Myra Falls					BOREHOLE NO: BH19-06			
DRILLER/DRILL: Mudbay Drilling/Soni	British Columbia, BC					PROJECT NO: NX14001C1.3						
METHOD: Sonic/iBPT Drilling	NORTHING: 5494677.8 EASTING: 311957.7					EL	EVATION: 362.4					
SAMPLE TYPE TUBE	NO RECO		SF			GRAB		LP		CORE		
BACKFILL TYPE BENTON	ITE BENTONI	ITE CHIPS	BE	NTONITE	PELLETS			L∎PI	EZOMETER HEAD	SAND		
	SOIL CRIPTION	SAMPLE TYPE		20 100 100 ▲ SPT* 40	—SCPT \ 200 —CPT q 200 N"▼LPT "N" 図iBP' 80 PLASTIC MC	/ _s (m/s) 300 (BAR) 300 T N(60) (BLOWS/3 120	400 400 000 mm) 160		ADDITION. INFORMAT		BACKFILL	ELEVATION (m)
sand to sandy, silty, grey sand to sandy, silty, grey GRAVEL, fine to coarse, occasional cobbles, sand brown/orange/grey (wast) 25 GRAVEL, fine to medium trace to some silt, grey (wast) GRAVEL, fine to medium trace to some silt, grey (wast)	angular to subangular, ly, some silt to silty, e rock fill)	24.4m 27.7m ndy,	- LPT1	20 17 11 11 14 14 14 15 16 16 16 16 16 16 16 16 16 16 16 16 16	40 				.05/3.05 m 1.52/1.53 m w Count60-75-65			341- 340- 339- 337- 336- 334-
Se souls M	Environment & Int	frastructu	ire Sc	olutions	LOGGED E	BY: HY			COMPLETION COMPLETION			
WOOD.	Burnaby,	BC V5C	0E4	~ <i>y</i>	REVIEWE				CONIFILETION			of 4
OL					· _ V _ V _ L	אום וים כ			1	ıa	g - 0	

CLIENT: Nyrstar Myra Falls Ltd.	PROJ	ECT:	Myra Falls	BOREHOLE NO: BH19-06					
			British Columbia, BC				PROJECT NO: NX14001C1.3		
METHOD: Sonic/iBPT Drilling	NORT			EASTING: 31	1957.7	ELE	EVATION: 362.4 m		
SAMPLE TYPE TUBE	✓ NO RECOVERY		SPT		GRAB	LPT			
BACKFILL TYPE BENTON	ITE BENTONITE CHI	IPS (BENTONIT	E PELLETS (GROUT	■ PIE	ZOMETER HEAD SAND		
	SOIL CRIPTION	SAMPLE TYPE	ON 10 10 10 10 10 10 10 10 10 10 10 10 10	0 40 -SCPT V _s (n 0 200 -CPT q. (BA 0 200 "N" ▼ LPT "N" ■ IBPT N(0 80 PLASTIC MC	300 400 (R) 300 400		ADDITIONAL INFORMATION	BACKFILL	ELEVATION (m)
-31			LPT2		1	C13: REC:0.19/0.	52/1.53 m Count 85/1.5" m		331-
sand to sandy, trace to so	35.4n subangular to subrounded, some ome silt,moist, brown/grey	<u> </u>				SLPT4: Blow REC:0.29/0.	Count 51-36-33		328
(colluvial, native) tow small layres of cler 36.3 m depth observed	aner sandy gravel at 36 m and					C15: REC:3.	05/3.04 m		326
subangular), trace to som native)	38.4n ned, gravelly (subrounded to le silt, moist, grey (colluvial, 39.3n ned, gravelly (subrounded to loist, grey (colluvial, native) 39.6n	n				Borehole gro cement bent Installed VW Initial readin Initial readin 12/3/2019 at	REHOLE AT 39.6 m buted from 39.6 m to ground surface wi onite grout 63328 at elevation 327.05 m g prior installation: 8643.0B, 12°C gs after installtion: 8520.5B, 11°C = 329.75 m 8666.9, 12.3°C = 326.61 m	ith	324
<u></u>	Environment & Infrastru	ıctur	re Solution	LOGGED BY	: HY	121012013 d	COMPLETION DEPTH: 39	9.6 m	
Wood.	600 - 4445 Loughee	ed Hi	ighway	ENTERED B	Y: HY		COMPLETION DATE: 12/3)
Ö	Burnaby, BC V	5C 0	E4	REVIEWED E	BY: BW		F	² age	4 of 4

wood.

Appendix D – RST Calibration Documents



Calibration Record

RST Instruments Ltd., 11545 Kingston St., Maple Ridge, British Columbia, Canada V2X 0Z5 Tel: 604 540 1100 • Fax: 604 540 1005 • Toll Free: 1 800 665 5599 (North America only) e-mail: info@rstinstruments.com • Website: www.rstinstruments.com

Vibrating Wire Piezometer

Customer:

Nyrstar Myra Falls Ltd

Model:

Serial Number:

VW2100-0.7

Mfg Number:

VW63328

Range:

P116287

Temperature:

700.0 kPa

23.0 °C

Barometric Pressure: Work Order Number:

1022.9 millibars

Cable Length:

222975

264399 m

168 meters 264567 m

Cable Markings:

Red / Black (Coil)

to

Cable Colour Code:

Green / White (Thermistor) EL380004

Cable Type: Thermistor Type:

3 kΩ

Applied	First	Second	Average	Calculated	Linearity	Calculated	Polynomial
Pressure	Reading	Reading	Reading	Linear	Error	Polynomial	Error
(kPa)	(B units)	(B units)	(Bunits)	(kPa)	(% FS)	(kPa)	(% FS)
0.0	8613	8614	8613	0.5	0.08	0.4	0.05
140.0	7956	7957	7956	139.5	-0.07	139.5	-0.06
280.0	7295	7295	7295	279.4	-0.09	279.6	-0.06
420.0	6628	6629	6628	420.3	0.05	420.5	0.07
560.0	5966	5966	5966	560.4	0.06	560.4	0.06
700.0	5307	5307	5307	699.8	-0.02	699.6	-0.05
			Max.	Error (%):	0.09		0.07

Linear Calibration Factor:

CF = 2.1149E-01 kPa/B unit

Temperature Correction Factor:

Tk = 9.0413E-02 kPa/°C rise

Polynomial Gage Factors:

 $A = -1.2838E-07 \text{ kPa/(B unit)}^2$

-2.0970E-01 kPa/B unit

kPa

Pressure is calculated with the following equations:

Linear:

 $P = CF(L_0 - L) - Tk(T_0 - T) + (S_0 - S)$

Polynomial:

 $P = A(L^2) + B(L) + C - Tk(T_0 - T) + (S_0 - S)$

Users must establish site zero readings for calculation purposes Polynomial C = - $[A(L_0^2) + B(L_0)]$

L₀, L = initial (installation) and current readings, in B units

To, T = initial (installation) and current temperature, in °C

S₀, S = initial (installation) and current barometric pressure readings, in kPa

B units = B scale output of VW 2102, VW 2104, VW 2106 and DT 2011 readouts

B units = $Hz^2/1000$

ie: 1700 Hz = 2890 B units

Date VW Reading Temperature Baro (dd/mm/yy) (B units) (°C) (mbar)

Shipped Zero Readings:

26-Nov-19

20.1

1012.0

This instrument has been calibrated using standards traceable to the NIST in compliance with ANSI Z540-1

Technician:

O. Nygren

Date: <u>26-Nov-19</u>





Calibration Record

RST Instruments Ltd., 11545 Kingston St., Maple Ridge, British Columbia, Canada V2X 0Z5 Tel: 604 540 1100 • Fax: 604 540 1005 • Toll Free: 1 800 665 5599 (North America only) e-mail: info@rstinstruments.com • Website: www.rstinstruments.com

Vibrating Wire Piezometer

Customer:

Nyrstar Myra Falls Ltd

Model:

VW2100-0.7

Serial Number:

VW63327

Mfg Number:

V VV 63327

Mfg Number: Range: P116288

Temperature:

700.0 kPa 23.0 °C

Barometric Pressure:

1022.9 millibars

Work Order Number:

222975

EL380004

Cable Length:

147 meters

Cable Markings:

264249 m

Red / Black (Coil)

264397 m

Cable Colour Code:

Green / White (Thermistor)

Cable Type: Thermistor Type:

3 кΩ

Applied	First	Second	Average	Calculated	Linearity	Calculated	Polynomial
Pressure	Reading	Reading	Reading	Linear	Error	Polynomial	Error
(kPa)	(B units)	(B units)	(B units)	(kPa)	(%FS)	(kPa)	(% FS)
0.0	9346	9345	9345	0.9	0.13	0.1	0.02
140.0	8695	8695	8695	139.8	-0.03	139.9	-0.01
280.0	8042	8043	8042	279.0	-0.14	279.6	-0.05
420.0	7383	7384	7384	419.7	-0.05	420.3	0.04
560.0	6726	6726	6726	560.0	0.00	560.2	0.03
700.0	6067	6068	6068	700.6	0.09	699.8	-0.02
Max. Error (%): 0.14							

Linear Calibration Factor:

CF = 2.1347E-01 kPa/B unit

Temperature Correction Factor:

Tk = 2.2868E-01 kPa/°C rise

Polynomial Gage Factors:

 $A = -5.2957E-07 \text{ kPa/(B unit)}^2$

B = -2.0531E-01 kPa/B unit

C = kP

Pressure is calculated with the following equations:

Linear:

 $P = CF(L_0 - L) - Tk(T_0 - T) + (S_0 - S)$

Polynomial:

 $P = A(L^2) + B(L) + C - Tk(T_0 - T) + (S_0 - S)$

Users must establish site zero readings for calculation purposes

Polynomial C = - [$A(L_0^2) + B(L_0)$]

 $L_{\text{\scriptsize 0}},\,L$ = initial (installation) and current readings, in B units

 T_0 , T = initial (installation) and current temperature, in °C

So, S = initial (installation) and current barometric pressure readings, in kPa

B units = B scale output of VW 2102, VW 2104, VW 2106 and DT 2011 readouts

B units = $Hz^2/1000$

ie: 1700 Hz = 2890 B units

Date VW Reading Temperature Baro (dd/mm/yy) (B units) (°C) (mbar)

Shipped Zero Readings:

26-Nov-19

9363

<u>19.9</u>

1012.0

This instrument has been calibrated using standards traceable to the NIST in compliance with ANSI Z540-1

Technician:

O. Nygren

Date: <u>26-Nov-19</u>



Calibration Record

RST Instruments Ltd., 11545 Kingston St., Maple Ridge, British Columbia, Canada V2X 0Z5 Tel: 604 540 1100 • Fax: 604 540 1005 • Toll Free: 1 800 665 5599 (North America only) e-mail: info@rstinstruments.com • Website: www.rstinstruments.com

Vibrating Wire Piezometer

Customer:

Nyrstar Myra Falls Ltd

Model:

VW2100-0.7

Serial Number:

VW63326

Mfg Number:

P116261

Range:

700.0 kPa

Temperature:

23.0 °C

Barometric Pressure:

1023.0 millibars

Work Order Number:

222975

Cable Length: Cable Markings:

802326 m

Red / Black (Coil)

85 meters 802411 m

Cable Colour Code:

to

Green / White (Thermistor)

Cable Type:

EL380004

Thermistor Type:

3 кΩ

Applied Pressure (kPa)	First Reading (B units)	Second Reading (B units)	Average Reading (B units)	Calculated Linear (kPa)	Linearity Error (% FS)	Calculated Polynomial (kPa)	Polynomial Error (% FS)
0.0	9596	9598	9597	1.3	0.18	0.2	0.03
140.0	8937	8938	8937	139.6	-0.06	139.8	-0.03
280.0	8273	8274	8273	278.8	-0.17	279.7	-0.05
420.0	7602	7603	7603	419.4	-0.08	420.3	0.05
560.0	6932	6933	6932	560.0	-0.01	560.2	0.03
700.0	6260	6260	6260	700.9	0.13	699.8	-0.03
			Max.	Error (%):	0.18		0.05

Linear Calibration Factor:

CF = 2.0966E-01 kPa/B unit

Temperature Correction Factor:

Tk = 2.0914E-01 kPa/°C rise

Polynomial Gage Factors:

 $A = _-7.3890E-07 \text{ kPa/(B unit)}^2$

-1.9794E-01 kPa/B unit

kPa

Pressure is calculated with the following equations:

Linear:

 $P = CF(L_0 - L) - Tk(T_0 - T) + (S_0 - S)$

Polynomial:

 $P = A(L^2) + B(L) + C - Tk(T_0 - T) + (S_0 - S)$

Users must establish site zero readings for calculation purposes

Polynomial C = - $[A(L_0^2) + B(L_0)]$

L₀, L = initial (installation) and current readings, in B units

T₀, T = initial (installation) and current temperature, in °C

S₀, S = initial (installation) and current barometric pressure readings, in kPa

B units = B scale output of VW 2102, VW 2104, VW 2106 and DT 2011 readouts B units = $Hz^2/1000$

VW Reading Temperature

Baro

ie: 1700 Hz = 2890 B units

Date (dd/mm/yy)

(B units)

(°C)

(mbar)

Shipped Zero Readings:

26-Nov-19

9631

20.0

1012.0

This instrument has been calibrated using standards traceable to the NIST in compliance with ANSI Z540-1

Technician:

O. Nygren

Date: 26-Nov-19

Document Number .: ELL0130K

wood.

Appendix E – SI Program Photographs



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BH19_01 DRILLING AND SEISMIC INVESTIGATION

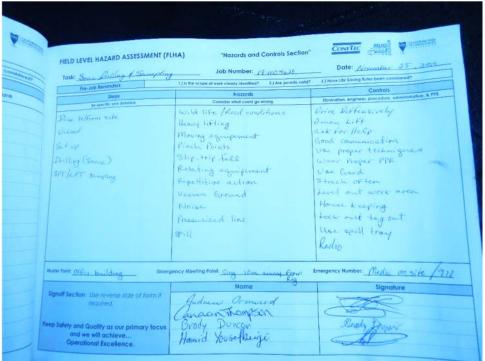


PHOTO 1: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 25, 2019)



PHOTO 2: BH19_01 Drilling Location on Lynx Dam (November 25, 2019)





2019 Site Investigation Photographs



PHOTO 3: BH19_01 Representative Sample Depth 15'-20' (November 25, 2019)



PHOTO 4: MASW19-02 Seismic Investigation (November 25, 2019)





BH19_01 DRILLING CONT' AND SI INSTALLATION, BH19_04 LOCATION, SEISMIC INVESTIGATION

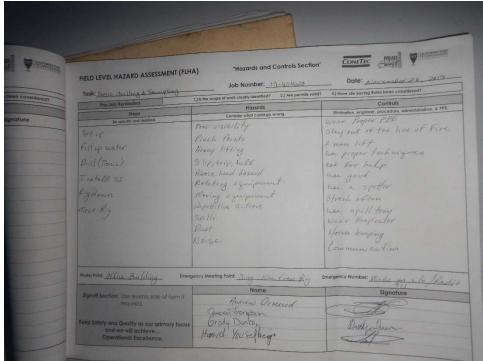


PHOTO 5: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 26, 2019)



PHOTO 6: BH19_01 Drilling Location on Lynx Dam (November 26, 2019)





2019 Site Investigation Photographs



PHOTO 7: BH19_01 Representative Sample Depth 80'-90' (November 26, 2019)



PHOTO 8: BH19_01 Representative Sample Depth 90'-100' (November 26, 2019)







PHOTO 9: Mifflin Surveys on Site (November 26, 2019)



PHOTO 10: BH19_01 SI Installation (November 26, 2019)





Scale: N/A Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Figure 5



PHOTO 11: Excavator Creating Path for MASW19-02 Seismic Investigation (November 26, 2019)



PHOTO 12: Excavator removing large rocks along Path for MASW19-02 Seismic Investigation (November 26, 2019)







PHOTO 13: BH19_01 SI Installation upon completion (November 26, 2019)



PHOTO 14: BH19_01 SI Installation upon completion (November 26, 2019)







PHOTO 15: BH19_04 Location (November 26, 2019)



PHOTO 16: BH19_01 Safety Barriers (November 26, 2019)





Scale: N/A Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Figure 8

BH19_04 AND BH19_02 DRILLING

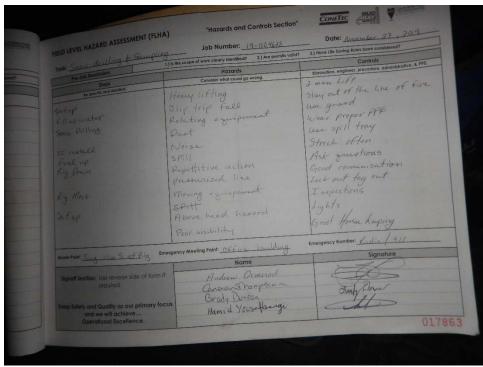


PHOTO 17: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 27, 2019)



PHOTO 18: BH19_04 Drilling Location on Lynx Dam (November 27, 2019)





2019 Site Investigation Photographs



PHOTO 19: BH19_04 Drilling Location on Lynx Dam (November 27, 2019)



PHOTO 20: BH19_04 Typical SI Casing Anchor (November 27, 2019)







PHOTO 21: BH19_04 Representative Sample Depth 140'-150' (November 27, 2019)



PHOTO 22: BH19_04 Safety Barrier placed upon completion (November 27, 2019)







PHOTO 23: BH19_02 Drilling Location (November 27, 2019)



PHOTO 24: BH19_02 Representative Sample Depth 20'-30' (November 27, 2019)





BH19_02 DRILLING

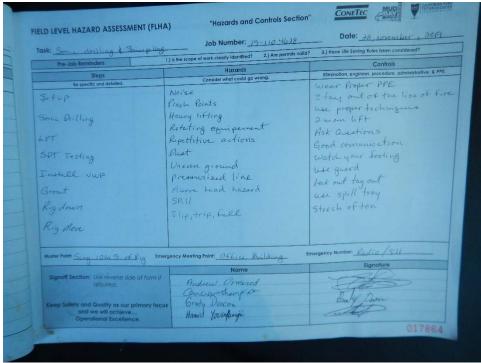


PHOTO 25: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 28, 2019)



PHOTO 26: BH19_02 Representative Sample (November 28, 2019)





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PHOTO 27: BH19_02 Soil Core Logs (November 28, 2019)



PHOTO 28: BH19_02 iBPT Refusal at 78' - 80' (November 28, 2019)







PHOTO 29: BH19_02 LPT Depth 95' - 95.5' (November 28, 2019)



PHOTO 30: BH19_02 LPT Depth 95' - 95.5' (November 28, 2019)







PHOTO 31: BH19_02 LPT Depth 105' - 106.5' (November 28, 2019)



PHOTO 32: BH19_02 LPT Depth 105' - 106.5' (November 28, 2019)





Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 16

BH19_02 DRILLING AND VWP INSTALLATION

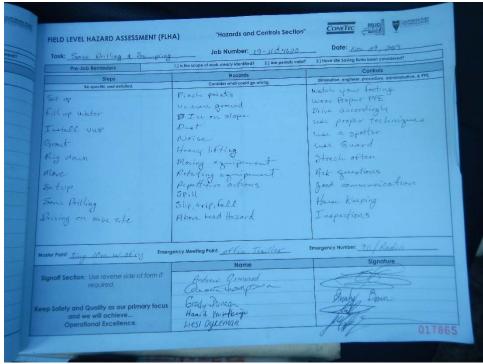


PHOTO 33: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 29, 2019)



PHOTO 34: BH19_02 VWP Installation (November 29, 2019)





2019 Site Investigation Photographs

Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 17



PHOTO 35: BH19_02 VWP Calibration Document (November 29, 2019)



PHOTO 36: BH19_02 VWP preparation (November 29, 2019)





Date: Nov. 2019 | Project: NX14001C1.3 Figure 18 Drawn: N/A Scale: N/A



PHOTO 37: BH19_02 VWP Installation (November 29, 2019)



PHOTO 38: BH19_02 Representative Sample Depth 10' - 20' (November 29, 2019)





Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 19



PHOTO 39: BH19_02 Representative Sample Depth 40' - 50' (November 29, 2019)



PHOTO 40: BH19_02 Representative Sample Depth 50' - 60' (November 29, 2019)





BH19_03 DRILLING

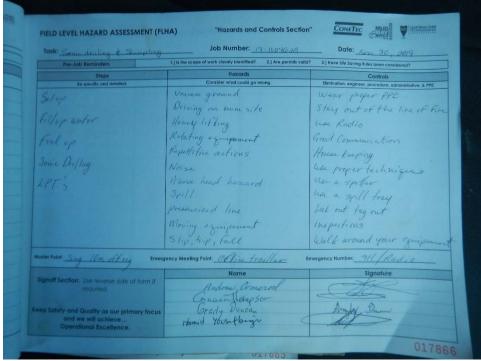


PHOTO 41: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (November 30, 2019)



PHOTO 42: BH19_03 Representative Sample Depth 60' - 70' (November 30, 2019)





2019 Site Investigation Photographs



PHOTO 43: BH19_03 Representative Sample Depth 70' - 80' (November 30, 2019)



PHOTO 44: BH19_03 LPT Sample Depth 80' - 81.5' (November 30, 2019)







PHOTO 45: BH19_03 LPT Sample Depth 80' - 81.5' (November 30, 2019)



PHOTO 46: BH19_03 Representative Sample Depth 85' - 90' (November 30, 2019)





Date: Nov. 2019 | Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 23



PHOTO 47: BH19_03 LPT Sample Depth 100' - 100.9' (November 30, 2019)



PHOTO 48: BH19_03 Representative Sample Depth 100' - 105' (November 30, 2019)





BH19_05 DRILLING, BH19_01 AND BH19_04 WELL INSTALLATION, BH19_03 VWP INSTALLATION

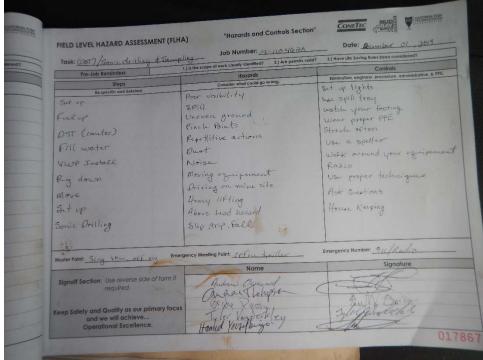


PHOTO 49: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (December 1, 2019)

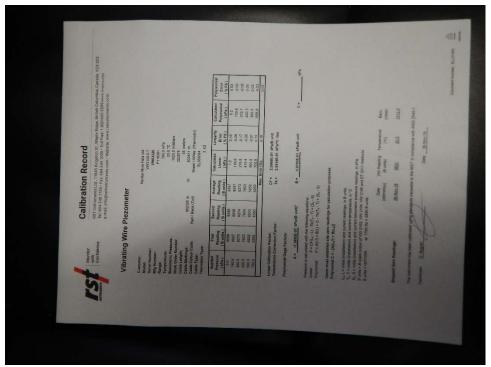


PHOTO 50: BH19_03 VWP Calibration Document (December 1, 2019)





2019 Site Investigation Photographs



PHOTO 51: BH19_03 VWP Initial readings prior to Installation (December 1, 2019)



PHOTO 52: BH19_04 Protective cover installation using bentonite chips (December 1, 2019)





Date: Dec. 2019 Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 26



PHOTO 53: BH19_04 Protective cover installation using bentonite chips (December 30, 2019)



PHOTO 54: BH19_05 Representative Sample Depth 10' - 20' (December 1, 2019)





Date: Dec. 2019 Project: NX14001C1.3 Drawn: N/A Scale: N/A Figure 27



PHOTO 55: BH19_01 Protective cover installation using bentonite chips (December 30, 2019)



PHOTO 56: BH19_05 Representative Sample Depth 90' - 91.5' (December 1, 2019)





BH19_05 AND BH19_06 DRILLING

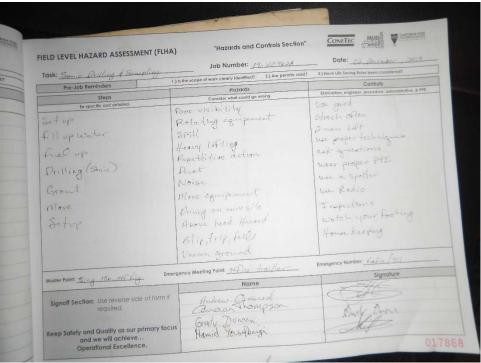


PHOTO 57: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (December 2, 2019)



PHOTO 58: BH19_05 Drilling Location (December 2, 2019)





2019 Site Investigation Photographs



PHOTO 59: BH19_05 LPT Sample Depth 95' - 96.5' (December 2, 2019)



PHOTO 60: BH19_05 Representative Sample Depth 100' - 110' (December 2, 2019)







PHOTO 61: BH19_06 Representative Sample Depth 100' - 110' (December 2, 2019)



PHOTO 62: BH19_06 Representative Sample Depth 100' - 110' (December 2, 2019)





BH19_06 DRILLING AND VWP INSTALLATION

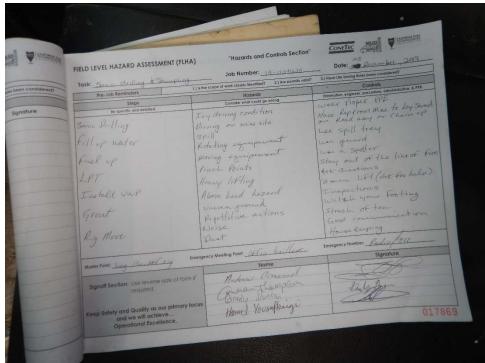


PHOTO 63: Signed ConeTec/Mud Bay Drilling Field Level Hazard Assessment Form (December 3, 2019)



PHOTO 64: BH19_06 VWP Installation (December 3, 2019)





2019 Site Investigation Photographs

Drawn: N/A Date: Dec. 2019 | Project: NX14001C1.3 Figure 32 Scale: N/A

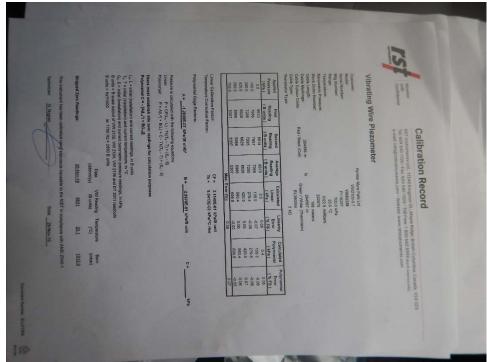


PHOTO 65: BH19_06 VWP Calibration Document (December 3, 2019)



PHOTO 66: BH19_06 LPT Sample Depth 95' - 96.5' (December 3, 2019)







PHOTO 67: BH19_06 LPT Sample Depth 95' - 96.5' (December 3, 2019)



PHOTO 68: BH19_06 Representative Sample Depth 95' - 100' (December 3, 2019)







PHOTO 69: BH19_06 Representative Sample Depth 120' - 130' (December 3, 2019)



PHOTO 70: BH19_06 Drilling Location (December 3, 2019)



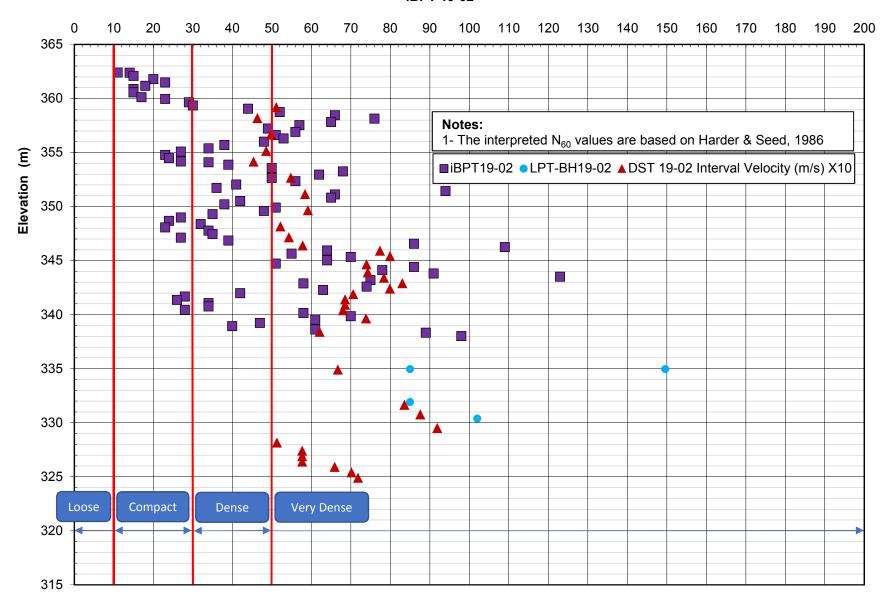


Drawn: N/A Date: Dec. 2019 | Project: NX14001C1.3 Figure 35 Scale: N/A

wood.

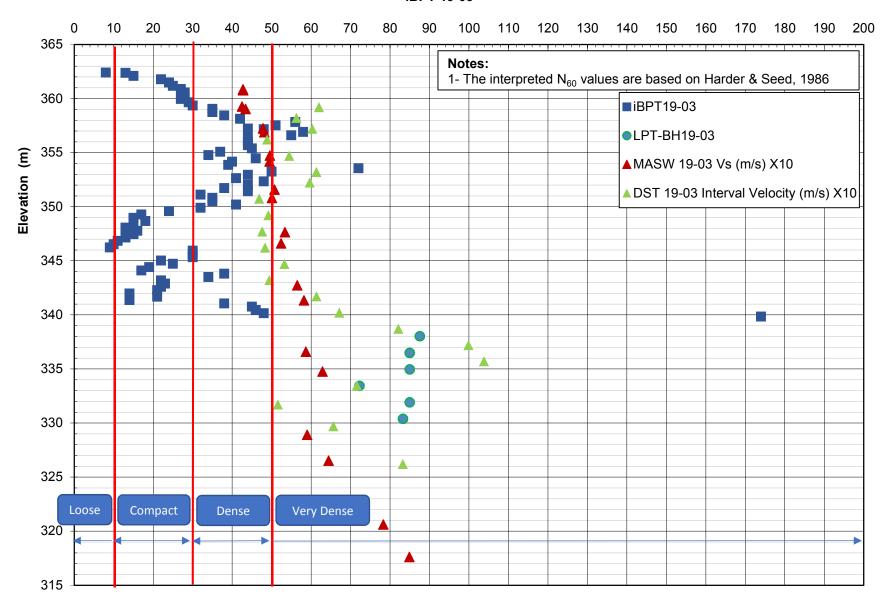
Appendix F – N₆₀ Vs. Elevation Plots

iBPT 19-02



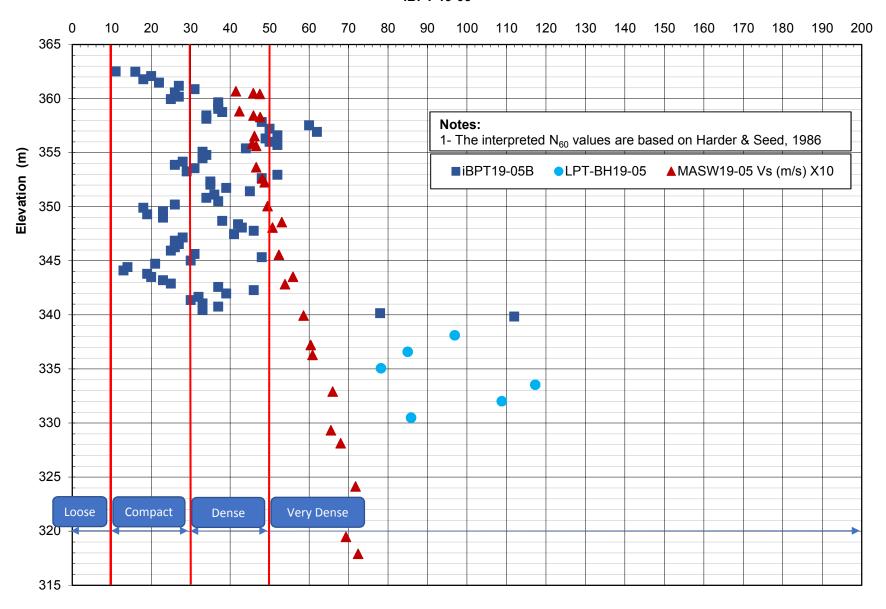
Equivalent Standard Penetration, N₆₀

iBPT 19-03



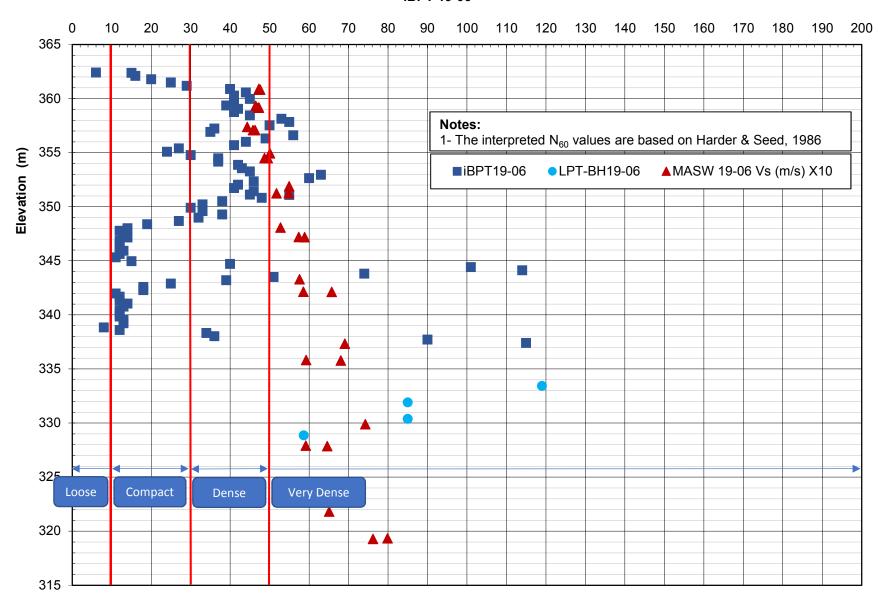
Equivalent Standard Penetration, N₆₀

iBPT 19-05



Equivalent Standard Penetration, N₆₀

iBPT 19-06



Equivalent Standard Penetration, N₆₀